

Basis of Design Report

LOS OLIVOS WASTEWATER HYBRID COLLECTION ANALYSIS

Prepared for: **Los Olivos Community Service District**

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EXECUTIVE SUMMARY

The purpose of this Basis of Design (BOD) is to compare alternative hybrid collection systems configuration and make an initial recommendation on the best approach for the unincorporated community of Los Olivos. This BOD has been conducted by Regen AEC (Regen) for the Los Olivos Community Service District (LOCSD) and the Los Olivos Wastewater Reclamation Program Project (LOWRPP). The City of Los Olivos has been analyzing solutions for the wastewater concerns for close over 15-years, during which time construction costs are estimated to have increased by roughly 85% based on the Construction Cost Index¹. With the current costs of inflation and the availability of funding the timing for a solution is critical, lest another 15 years of costs drive the price of alternatives even higher.

Within this analysis the community was divided into 6 zones based on guidance from the Board of Directors (BOD) of the Los Olivos Community Service District. The zones were utilized to evaluate three alternatives, including gravity sewer wastewater collection, effluent sewer wastewater collection, and advanced onsite individual on lot treatment and dispersal systems. Four options (A, B, C, & D) were analyzed utilizing these three alternative systems; Option A included a gravity sewer for the collection of wastewater within zones 1 & 2 (commercial area) and effluent sewer throughout zones 3, 4, 5, & 6; Option B included a effluent sewer throughout all zones; Option C included a gravity sewer for the collection of wastewater within zones 1 & 2 (commercial area), effluent sewer throughout zones 3, 4, & 5, and advanced onsite on lot systems throughout zone 6; Option D included a effluent sewer throughout zones 1, 2, 3, 4, & 5, and advanced onsite on lot systems throughout zone 6. The analysis included an evaluation of capital costs and wastewater constituents associated with all zones and options. A summary of the results is provided below:

Option	Avg Day	Max Month	Max Day	Peak Hour
Option	(gpd)	(gpd)	(gpd)	(gpm)
A	96,181	110,608	134,653	215
В	96,181	110,608	134,653	134
С	81,381	93,588	113,933	194
D	81,381	93,588	113,933	113

Table 1 – Wastewater Flow

Table 2 – Wastewater Constituents

04	Avg BOD	Avg TSS	Avg TKN
Option	(mg/L)	(mg/L)	(mg/L)
A	180	143	53
В	150	40	65
С	186	162	51
D	150	40	65

¹ https://www.mortenson.com/cost-index



Table 3 – Capital Costs

Option	Collection System Subtotal (\$US)	Advanced Onsite Subtotal (\$US)
A	\$25,503,016	\$0
В	\$21,637,492	\$0
С	\$23,064,728	\$6,734,00
D	\$18,669,808	\$6,734,00

Based on this analysis the economic benefits of an effluent sewer wastewater collection system option appear to make it the most viable solution at this moment. In addition to the economic benefits of the collection system there are additional benefits to the price, size, and complexity of the centralized treatment and reuse facility with use of an effluent sewer.



INTRODUCTION

This Basis of Design (BOD) report had been developed to provide the Los Olivos Community Service District (CSD or District) with a foundational design basis for the development of a hybrid wastewater collection system design. Regen has been contracted to assist the Los Olivos Community Services District with the design of a hybrid wastewater collection system.

STUDY AREA CHARACTERISTICS

The Los Olivos area is located within Santa Barbara County off California highway 154. The proposed wastewater collection area consists of 396 parcels and roughly 840 residents. Per adopted Resolution 2019-04, the Los Olivos Wastewater Reclamation Program Project (LOWRPP) was initiated to help identify strategies to provide viable wastewater collection and treatment for the residents and business owners within the District.



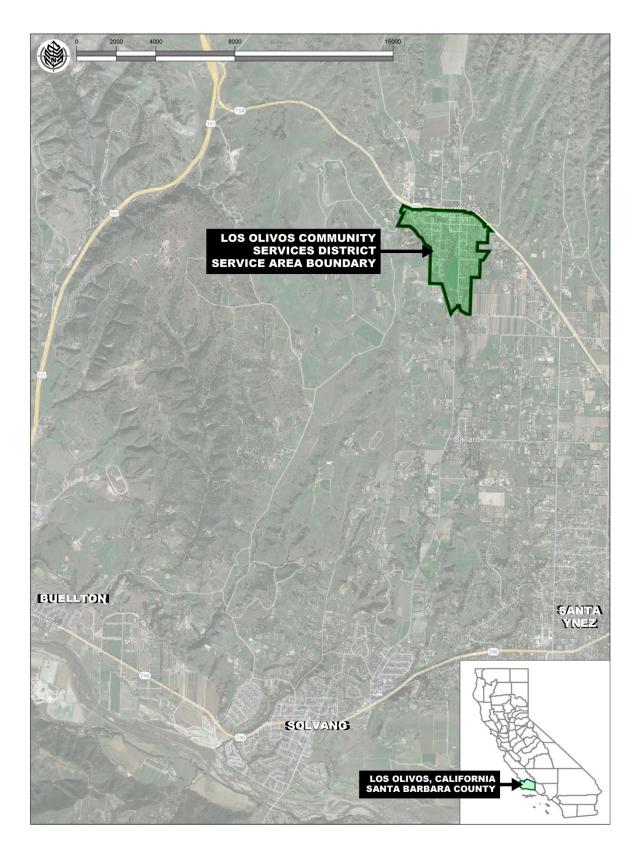


Figure 1 – Vicinity Map



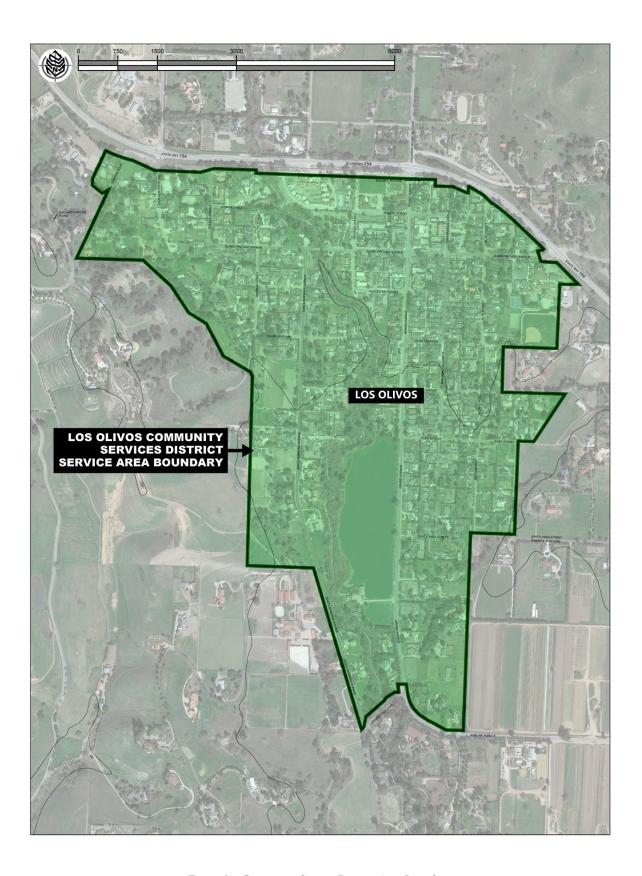


Figure 2 – Community Service District Area Boundary



PREVIOUS STUDIES

The community of Los Olivos has had several studies conducted over the past two decades to evaluate wastewater alternatives to address groundwater quality concerns. Key previous studies include:

- 1. Santa Ynez Valley Community Plan Environmental Impact Report (EIR 2009)
- 2. Los Olivos Wastewater Management Plan (LOWWMP 2010)
- 3. Los Olivos Wastewater System Preliminary Engineering Report (AECOM 2013)
- 4. Los Olivos Special Problems Area Sewer Calculations (Stantec 2015)
- 5. Final Draft Plan for Services and Feasibility Study (Berkson 2016)
- 6. Update to Los Olivos Wastewater System Preliminary Engineering Report (AECOM 2016)
- 7. Desktop Study- Proposed Wastewater Treatment Plant Siting Study (UPC 2021)
- 8. Septic to Sewer Task Order No. 1
- 9. Wastewater Collection and Treatment Basis of Design Report (Stantec 2022)



BASIS OF PLANNING

District Recommended Zone Area Boundaries

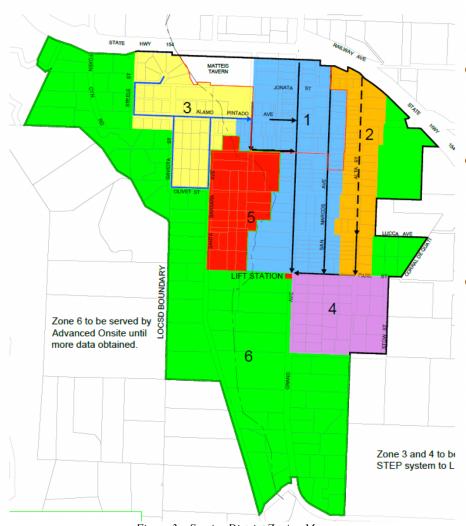


Figure 3 – Service District Zoning Map

Proposed Zone Collections System Alternatives

Collection system alternatives traditionally evaluated for residential development include gravity centralized lift stations, effluent collection systems (also known as step or liquid only sewers), and grinder systems.

Gravity sewers utilize large diameter lines, which gravity flow to a centralized location for further conveyance to a wastewater treatment facility. In Effluent Sewer system, the effluent (primary treated liquid) is typically pumped from the septic tank under low to medium pressure to a small-diameter, pressurized collector sewer. In the individual grinder lift stations, household sewage is collected in a small basin where the solids are macerated and then conveyed into transport lines with the grinder pumps. Residential step and grinder systems consist of an electrical panel, tank or basin, pump vault containing a single pump and level control.



Proposed Treatment Plant Locations

The wastewater treatment works are outside of the scope of Regen's collection system design work.

Treatment plant capacities are based on estimated flows from all residential and commercial properties. Estimated residential flows of 200 gpd average have been assumed, commercial property flows have been estimated based on water records and potential growth.

Hydraulic analysis will be based on the approved configuration when determined.

COLLECTION SYSTEM		

Gravity Collection

Conventional gravity sewage collection systems are the oldest forms of sewage collection and sanitation dating back to the Roman Empire. These systems generally require no mechanical or electrical facilities and rely solely on gravity to transport sewage from the points of connection to a central receiving location, either a transfer lift station or a wastewater treatment plant (WWTP). Gravity collection systems are designed with network of pipes placed at slopes sufficient to maintain minimum velocities to transport solids and prevent deposition and accumulation of materials in the system. Typically, the network is subdivided into primary pipes (sewer mainlines along main roads), secondary pipes, and tertiary pipes collecting wastes from individual neighborhoods and properties.

Design Criteria

Gravity sewer wastewater contains human waste solids, grit, and other solids that down the drain. In considering the solid content in gravity sewers they must be designed to "self-clean" which requires specific velocities to be maintained to "flush" the solids to their destination. Velocities must be maintained at a minimum of 2 to 3 fps (feet per second) to ensure minimal build-up of material within collection lines. Velocities are maintained by designing gravity sewer collection lines to have slopes as is related of flow and pipe diameter.

Manholes are required for access at given straight distances along the gravity sewer lines, at pipe intersections, and at any change in pipe direction. Manholes allow for maintenance, inspection, and cleaning of the gravity collection system. Manholes are generally required at the end of each line, at all changes in grade, size, or alignment, at all intersections, and at distances not to exceed 400 ft for sewers 15" or less (Recommended Standards for Wastewater Facilities, 2004).

Minimum pipe diameters are required in gravity sewers to minimize blockages and allow for adequate space for cleaning equipment and cameras. Although the District does not currently have standard design criteria established for gravity sewer collection systems there are standards set forth in the industry and by local agencies that will be the basis for design. Based on agency and industry standards (and previous studies), a 6-inch minimum gravity sewer main line will be utilized. Gravity sewer pipe materials are assumed to be either PVC SDR3-35 or HDPE PE3408.

Manning's equation for open channel flow will be utilized with a minimum allowable pipe slope and coefficient "n" equal to 0.013, where "n" is the roughness coefficient of the pipe material.



Pipe Size (inches)	Minimum Slope ¹ (%)	Maximum Liquid Depth to Diameter Ratio (d/D)	Maximum Percent Full (%)
8	0.4	0.5	50
10	0.28	0.5	50
12	0.22	0.5	50
15	0.16	0.75	75

Table 4 – Gravity Sewer Main Slopes and Design Depths

Estimated minimum cover over gravity sewer pipes should be maintained at 5 feet with an additional 1-foot vertical separation from existing or future utilities.

Lateral Connection Requirements

Each individual property will be required to connect to the gravity sewer collection system (where appropriate) with private laterals. Laterals are typically owned and maintained by the individual property owners. Each property owner is expected to be responsible for the construction of the lateral connection. Laterals are typically a minimum of 4-inch diameter at a minimum of 2 percent slope per the California Plumbing Code.

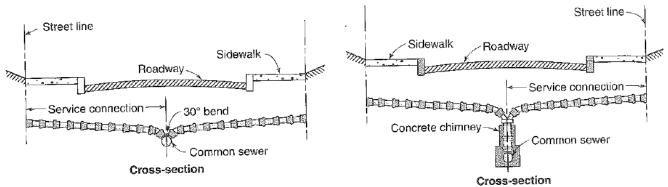


Figure 4 - Typical gravity sewer service lateral

Constructed costs for gravity sewers service laterals vary based upon main line depth, geological conditions, groundwater elevation, pipe material, and service lateral length.

Lateral size serving commercial multiple family dwellings must maintain pipe slope uniform from the sewer main to the property line. Minimum depths for laterals shall be maintained at 4 feet. Wye branches are used for lateral connections to mainline connections. Cleanouts shall be required with all lateral connections.

Right-of-way Requirements

ROW equipment for gravity sewers consists of large diameter mainline laid at a constant slope, manholes, lift stations (if required), and air release valves (if required). Costs fluctuate based upon bedding material, location (rural versus urban), clearing costs, topography, geological conditions, depth, and surface restoration costs. Table 2 excludes manholes, lift-stations, service wye's, and terminal cleanouts. It also assumes ideal soil conditions, no dewatering, and an 8-ft mean burial depth.

¹ Table 5.1 2013 AECOM Report



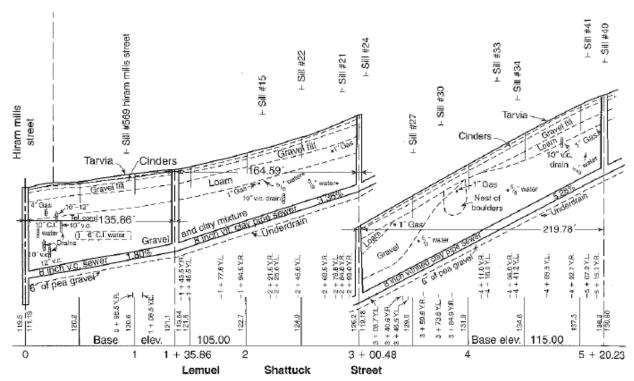


Figure 5 - Profile of gravity sewer

Table 5 - Installed Unit Costs: Gravity Sewer Pipe USD/Linear Ft (PVC).²

Item	Cost (2022 USD)
6" dia Mainline (USD/linear ft)	\$180
8" dia Mainline (USD/linear ft)	\$240
10" dia Mainline (USD/linear ft)	\$300

² Stantec, <u>Preliminary Construction Cost Estimate</u>, Los Olivos (28-Jun-22), WWTP South Side of District

Sewage Lift Station

When gravity sewers are installed in trenches deeper than 10 ft, the cost of sewer line installation increases significantly because of the more complex and costly excavation equipment and trench shoring techniques required. Lift stations are used to reduce mainline installation depth and, in some cases, reduce the capital cost of sewer system construction. Lift station construction has a significant economy of scale and is generally expensive and difficult to apply to small communities. For example, if the capacity of a lift station is increased by 100%, the construction cost would increase only by 50 to 55%.

A sewage lift station will be required to convey wastewater from the District gravity sewer collection system to a wastewater treatment plant regardless of the plant location. For estimating purposes, it is assumed the lift station will include a round manhole wet well, duplex submersible pumps, and telemetry controls. The lift station will need to include an odor control system.

The force main from the pump station to the WWTP shall be a dual force main to provide redundancy and reliability. It shall be two 6-inch diameter force mains, to be confirmed during design (Stantec 2022).



Effluent Sewer Collection

Effluent sewers utilize small settling tanks with pumping filters and effluent filtration units, small diameter transport lines (typically 2"-6") buried with the contours of the land just below frost depth, and small simple cleaning and air release ports throughout the pressurized line network. Since solids in an effluent sewer system are collected and digested in the on-lot tank, only liquid effluent is conveyed to the collection system. Line cleanings are eliminated as a result, so effluent sewer owners and users should be exempt from the charges typically associated with cleaning activities. In addition, effluent sewer collection systems are watertight, reducing infiltration costs in both conveyance and at the treatment facility.

Design Criteria

Transporting wastewater from the primary tanks to the centralized treatment facility will be accomplished with a 2"-4" force main lines. Assumptions include Hazen-Williams C-Factor of 150 and estimated flows per EDU (180 gpd, 3.5 people/dwelling unit).

The force mains in the conveyance system are typically only a few feet deep; therefore, there is potential of breakage due to future excavation events. Location wire and route markers will be used and strict enforcement controlling excavations in proximity of pipe should be exercised. Still, damage can occur and the used of isolation valves can be critical. Odor issues are a potential if primary tanks are not properly installed. All equipment should be installed to ensure proper seal of lids.

Because effluent sewers provide primary treatment on-lot and convey primary-treated and clarified effluent through a watertight, pressurized collection system that's largely immune to infiltration and inflow, they allow bioreactor volume reductions compared to other collection systems (gravity or grinder).

On Lot Processing Tank

A primary septic, or interceptor, tank will collect and retain raw sewage from each dwelling. In the interceptor tank, heavy solids (known as sludge) settle to the bottom while the lighter material (known as scum) floats to the top of the liquid contents. The organic material at the bottom of the tank (sludge) undergoes facultative and anaerobic digestion converting the organic matter to gases. Facultative microbes solubilize the complex organic material to volatile organic acids while strict anaerobes ferment the volatile organic acids to gases (methane, carbon dioxide, etc.). The rate at which both scum and sludge accumulates decreases as the biological process in the tank matures. It allows sufficient storage capacity for sludge and scum, resulting in long intervals between septage pump-outs. With long solids retention times, the tanks provide natural digestion, greatly reducing the impact of solids on a treatment facility. An effluent filter prevents any solids larger than 1/8-inch from reaching the pump. Typically, 40% to 60% of the BOD will be removed in the interceptor tank. It provides enough reserve space for 24 to 48 hours of normal operation before an emergency condition must be corrected, which minimizes the need for immediate maintenance. It provides an operating zone sufficient for modulating peak inflows without causing nuisance alarms or excessive hydraulic gradients.

The tanks in effluent sewers provide passive, long-term anaerobic digestion of primary sludge, flow equalization internal to the collection system, resistance to infiltration and inflow (I/I), and fine-screened effluent to the wastewater treatment facility. They facilitate the downsizing or complete elimination of influent fine-screen processes directly upstream of any wastewater secondary treatment process. The lower organic load of effluent sewers and their near elimination of I/I also permit smaller bioreactors (up



to $\sim 57\%$ smaller), reduce bioreactor aeration requirements (lowering bioprocess aeration by $\sim 57\%$), and reduce biosolids management demands (by up to 75%).

For smaller clustered units such as the cottages and villas, and for commercial buildings such as retail shops and offices, one interceptor tank may serve more than one building. Tanks will be furnished appropriately sized for the expected waste flows, typically at a minimum of 2.2 times design flow. For larger users, such as the restaurant and clubhouse, two tanks in series may be used. Grease traps will be required for all commercial kitchen facilities.

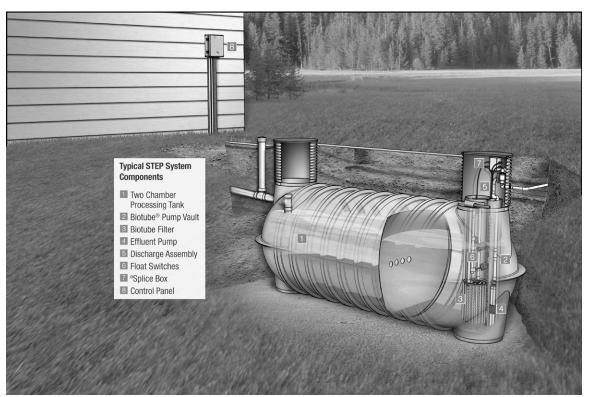


Figure 6 - Typical STEP system components.

courtesy of Orenco Systems Inc.

Lateral Connection Requirements

Effluent sewers use watertight tanks and low-pressure sewer mains. The mains are also watertight and do not include manholes; therefore, they are largely resistant to I/I. Per capita average flows are typically 50 gpcd.

Right-of-way Requirements

Mainline and appurtenances for pressure sewers typically consist of small diameter mainlines (2" to 6" typical), service saddles, air release valves, clean-outs, pigging ports, and mainline isolation valves. Effluent sewer lines are typically installed at minimum depths of 24" to 30" or below frost depth and follow the contour of the land. Mainline material is generally polyvinyl chloride (PVC), polyethylene (PE or HDPE), with pipe buried at shallow depths and with fewer joints compared to gravity sewer due to their increased individual pipe lengths.



Water Lateral Separation Requirements

Individual lot water lateral separation will be required at a minimum of 5 horizontal feet. Primary tank separation from main water lines shall be maintained at a minimum of 25 horizontal feet.

WASTEWATER TREATMENT FACILITY SITING

The siting and size of the treatment facility is outside of the scope of current contract; however, assumptions were made based on previous engineering work. Based on the Stantec Basis of Design Report the preliminary WWTP site will need a minimum of 1.6 acres to accommodate the treatment process, influent/effluent storage, truck access, equipment, buildings/screening, and other onsite needs at buildout of the facility. In addition to the two top recommendations from the Stantec report, an additional option to utilize land at the school was discussed as an alternative treatment and dispersal of effluents. This cooperative with the school district would have to be met before moving forward with additional analysis. For the purpose of this analysis a treatment facility located at the South point of the District was utilized for sizing of collection lines. Based on a general hydraulic analysis a location to the North or at the School would not dramatically impact the cost of the effluent sewer collection system.

ADVANCED ONSITE SYSTEMS

Advanced onsite systems may be an alternative for larger lots within the district. Advanced onsite systems are individual lot systems that collect, treat, and disperse treated water to an on-lot soil dispersal system. Based on the expected background aquifer nitrogen levels it was assumed that standard septic systems would not be an alternative within the district, additional it has been assumed that nitrogen specific advanced onsite systems would be required if they were identified as a feasible solution. Additional evaluation is required to verify that advanced onsite systems are an approvable solution within the district.

Advanced onsite systems include a primary solids settling tank, aeration process for secondary treatment, nitrogen reduction specific processes, and a soil absorption system for final dispersal of treated water. There are a variety of treatment processes and manufacturers available in the area that have systems that can meet the needs of the regulatory requirements if identified as a feasible alternative.



WASTEWATER COLLECTION SYSTEM OPTIONAL LAYOUTS

Various layouts were analyzed based on the recommended district zone map. Options A, B, C, and D provide alternative configurations that can be evaluated as additional information materializes with respect to systems costs and potential for approval of individual on-lot advanced treatment systems.

Option A - Gravity Sewer in central town (zones 1 & 2), Effluent Sewer in area surrounding downtown (zones 3-6)

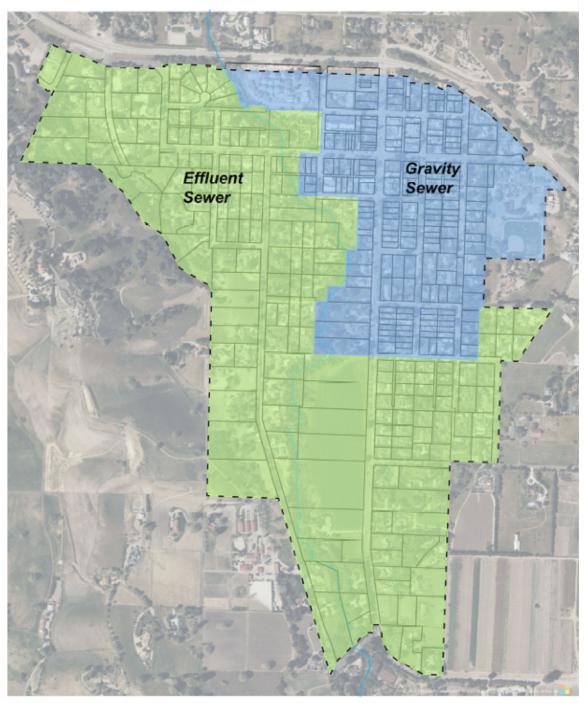


Figure 7 – Proposed Hybrid Sewer Collection System Alternative A



Option B - Effluent Sewer for entire district (zones 1-6)

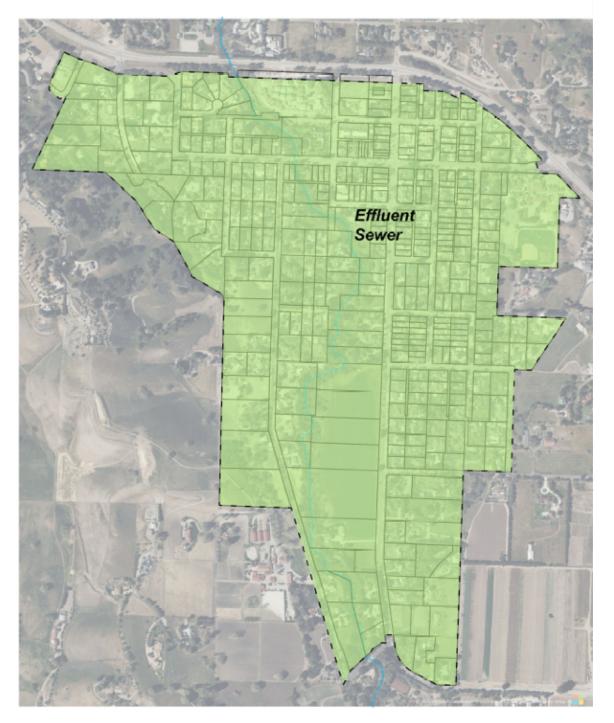


Figure 8 – Proposed Hybrid Sewer Collection System Alternative B



Option C - Gravity Sewer in central town (zones 1 & 2), Effluent Sewer in immediate area surrounding downtown (zones 3-5), Advanced Onsite Systems (zone 6)

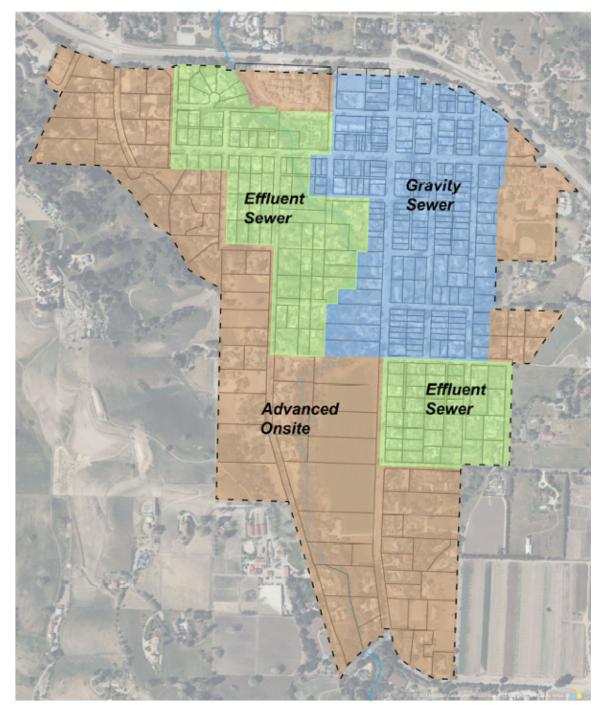


Figure 9 – Proposed Hybrid Sewer Collection System Alternative C



 ${\color{red}Option \ D - \textit{Effluent Sewer in dense areas (zones 1-5), Advanced Onsite Systems (zone 6)}}$

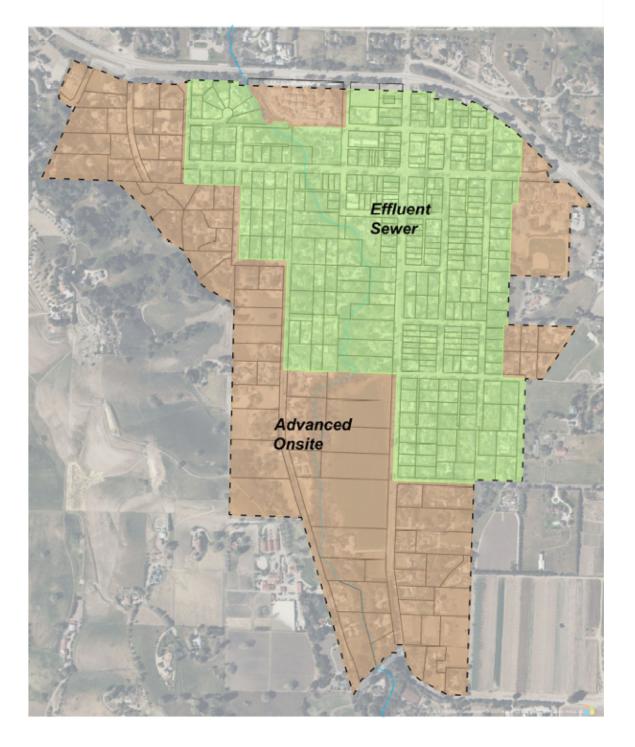


Figure 10-Proposed Hybrid Sewer Collection System Alternative D



WASTEWATER FLOW AND COMPOSITION ANALYSIS

Wastewater collection options such as Gravity, grinder, and effluent sewers each deliver unique hydraulics and wastewater characteristics to downstream wastewater treatment facilities that greatly affect the design, capital costs, performance, and operational costs of treatment facilities.

The type of wastewater collection system also influences primary and biosolid accumulation and management requirements at treatment facilities. Effluent sewers, when paired with MBRs, reduce the overall volume of primary solids and waste-activated sludge generated by up to 75% (refer to Figure 15 and Table 7 for additional details). With the trend for more stringent regulations governing the disposal of solids, design options that reduce the overall quantity of solids warrant close attention.

Zone Populations

Equivalent **Population Parcels Dwelling** Equivalent Zone Units (2.5/EDU)(EDU's) 1 149 249 623 2 58 58 145 3 46 46 115 4 43 43 108 5 26 26 65 6 74 74 185 **Total** 396 496 1240

Table 6 – Zone Populations

Alternative Collection System Typical Loading Rates

Table 7 – Alternative Collection System Typical Loading Rates

Constituent Loading Assumptions	Effluent Sewer	Grinder Sewer	Gravity Sewer
Design Average Flow	50 gpcd	50 gpcd	120 gpcd
Biochemical Oxygen Demand (BOD ₅)	150 mg/L	450 mg/L	200 mg/L
Chemical Oxygen Demand (COD)	381 mg/L	1143 mg/L	508 mg/L
Total Suspended Solids (TSS)	40 mg/L	500 mg/L	210 mg/L
Total Kjeldahl Nitrogen (TKN)	65 mg/L	70 mg/L	35 mg/L
Ammonia (NH ₃ -N)	40 mg/L	55 mg/L	21 mg/L
Total Phosphorus	16 mg/L	17 mg/L	7 mg/L
Fats, Oils, Greases (FOG)	15 mg/L	164 mg/L	80 mg/L

¹Adapted from Metcalf & Eddy 2003; Crites and Tchobanoglous 1998; USEPA 2002; Winneberger 1984.



Table 8 – Estimated Typical Flow Rates

Zone	Effluent Sewer Avg Day	Grinder Sewer Avg Day	Gravity Sewer Avg Day
	(gpd)	(gpd)	(gpd)
1	31,150	31,150	74,760
2	7,200	7,200	17,400
3	5,750	5,750	13,800
4	5,400	5,400	12,960
5	3,250	3,250	7,800
6	9,250	9,250	22,200
Total	62,050	62,050	148,920

Gravity Wastewater Hydraulic and Constituents Estimates

Table 9 – Gravity Collection Hydraulic Estimates

Zone	Avg Day* (gpd)	Max Month (gpd)	Max Day (gpd)	Peak Hour (gpm)	Peak Hour Factor
1	46,781	53,798	65,493	130	4
2	11,600	13,340	16,240	32	4
Total	58,381	67,138	81,733	162	4

^{*} Average day flow based on current water records

Table 10 – Gravity Collection Wastewater Constituent Estimates

Contaminant	Typical Composition	Design Values
Total Suspended Solids (TSS)	175 to 300 mg/L	200 mg/L
BODs at 20°C	200 to 350 mg/L	210 mg/L
Nitrogen (total as N)	30 to 70 mg/L	45 mg/L
Phosphorous (total as P)	6 to 12 mg/L	7 mg/L

Effluent Sewer Wastewater Hydraulic and Constituent Estimates

Table 11 – Effluent Sewer Hydraulic Estimates

Zone	Avg Day*	Max Month	Max Day	Peak Hour	Peak Hour Factor
	(gpd)	(gpd)	(gpd)	(gpm)	Factor
1	46,781	53,798	65,493	65	2
2	11,600	13,340	16,240	16	2



3	9,200	10,580	12,880	13	2
4	8,600	9,890	12,040	12	2
5	5,200	5,980	7,280	7	2
6	14,800	17,020	20,720	21	2
Total	96,181	110,608	134,653	134	2

^{*} Average day flow based on current water records

Table 12 – Effluent Sewer Collection Wastewater Constituent Estimates

Contaminant	Typical Composition	Design Values
Total Suspended Solids (TSS)	35 to 50 mg/L	40 mg/L
BODs at 20°C	110 to 220 mg/L	150 mg/L
Nitrogen (total as N)	40 to 70 mg/L	65 mg/L
Phosphorous (total as P)	8 to 18 mg/L	16 mg/L

Options Wastewater Hydraulic Load Estimates

Table 13 – Option Hydraulic Estimates

Option	Avg Day (gpd)	Max Month (gpd)	Max Day (gpd)	Peak Hour (gpm)
A	96,181	110,608	134,653	215
В	96,181	110,608	134,653	134
С	81,381	93,588	113,933	194
D	81,381	93,588	113,933	113

Options Wastewater Constituent Load Estimates

Table 14 - Option Biological and Solids Loading Estimates

04	Avg BOD	Avg TSS	Avg TKN
Option	(mg/L)	(mg/L)	(mg/L)
A	180	143	53
В	150	40	65
С	186	162	51
D	150	40	65

Wastewater Flow and Composition Summary

Options B & D provide both flow and composition benefits to the design of the centralized treatment facility and reuse or treated water discharge systems. The reduced peak hydraulic capacity and reduced wastewater constituents are expected to reduce the capital costs associated with the treatment facility.



SUMMARY

Based on the analysis above and attached estimated construction costs for each option, the lowest capital cost options effluent sewer technology coupled with advanced onsite initial options for zone 6. Effluent sewer collection technology reduces waste strength and hydraulic loads, compared to gravity sewer options, in a manner that is energy-conscious, environmentally sustainable, and cost-efficient. Effluent sewers also allow for a reduction in biosolids handling costs and eliminate sewer line cleaning.

Preliminary Cost Summary

Small communities face enormous challenges when constructing and maintaining wastewater infrastructure. Conventional collection system technologies — when applied to small, rural communities — typically result in costs that exceed affordability thresholds and ultimately require grant subsidies to attain reasonable user rates.

Option	Overhead and Construction	Gravity Sewer Construction	Effluent Sewer Construction	Collection Contingency Costs	Advanced Onsite Construction	Engineering Costs	Costs Provided by District
	(\$US)	(\$US)	(\$US)	(\$US)	(\$US)	(\$US)	(\$US)
A	\$2,830,000	\$6,777,000	\$3,866,382	\$4,042,015	\$0	\$5,254,619	\$2,760,000
В	\$2,830,000	\$0	\$8,279,524	\$3,332,857	\$0	\$4,332,714	\$2,360,000
С	\$2,830,000	\$6,777,000	\$2,407,632	\$3,604,390	\$6,734,000	\$4,685,706	\$2,760,000
D	\$2,830,000	\$0	\$6,820,774	\$2,895,232	\$6,734,000	\$3,763,802	\$2,360,000

Table 15 – Cost Estimates Breakdown

Table 16 – Cost Estimate Totals

Option	Collection System Subtotal	Advanced Onsite Subtotal
	(\$US)	(\$US)
A	\$25,503,016	\$0
В	\$21,637,492	\$0
С	\$23,064,728	\$6,734,00
D	\$18,669,808	\$6,734,00

Alternative collection systems were developed and designed to avoid the shortcomings associated with applying gravity sewers to small communities. Historically, effluent sewers (\$9,702/connection) have resulted in an average cost savings of \$6,692 (41%), when compared to gravity sewers (\$16,394/connection). In California the price of construction and material greatly exceed costs seen throughout the country, yet the savings historically seen with effluent sewer installations appear to still hold true. Effluent sewers may also offer for expedited installation times within the right-or-ways as



small diameter pipes and directional boring activities are not as extensive as trenching of deep gravity sewer lines.

Additional analysis should be completed to further evaluate hydraulic loads for the various alternatives as flows from water records may not accurately reflect flows associated with gravity sewers where groundwater impact may impact the flow a wastewater treatment facility receives from any gravity connection. Additionally, wastewater characteristics from gravity sewers are estimated assuming impacts from groundwater and additional infiltration and inflow sources, therefore wastewater characteristics are based on the 120 gpd per capita typical flow.

An effluent sewer alternative would also all for further evaluation in connecting to Solvang as they would likely not require additional booster stations to transport the effluent.

Alternatives C&D provide a phased approach which includes the use of Advanced Onsite Systems throughout zone 6, which allows for a slightly reduced capital cost for both collection and treatment. It should be noted that the costs associated with the Advanced Onsite Systems includes treatment and dispersal, compared to the Effluent Sewer and Gravity costs which only include collection (treatment and dispersal or reuse will be additional cost evaluated in another report). If one of these two options is selected for funding and construction, there is a future opportunity for advanced onsite systems to remain in operations while connecting to the centralized facility. This future option would greatly reduce the need for the centralized treatment facility to expand based on organic loads, but simply require the need for additional hydraulic capacity, which could drastically reduce the cost for future expansion.

Based on the economic estimates, and the potential reduction of community disturbances during construction it is recommended that Option B & D be considered for funding and further design as the most viable collection alternative for the community of Los Olivos. These two alternatives would also all for further evaluation in connecting to Solvang as they would likely not require additional booster stations to transport the effluent.



APPENDIX A

30% DESIGN PLANS FOR:

LOS OLIVOS **CALIFORNIA**

EFFLUENT SEWER WASTEWATER COLLECTION SYSTEM

DRAWING INDEX

SHEET
NIII 485

NUMBER SHEET NAME

C000	TEMPORARY
C101	COVER SHEET
C102	SYSTEM OVERVIEW
C103	ZONES 1-3
C104	ZONES 4-5
C105	70NF 6

EXAMPLE OM LOT TANK DETAILS EFFLUENT SEWER DETAILS EFFLUENT SEWER DETAILS

LEGEND

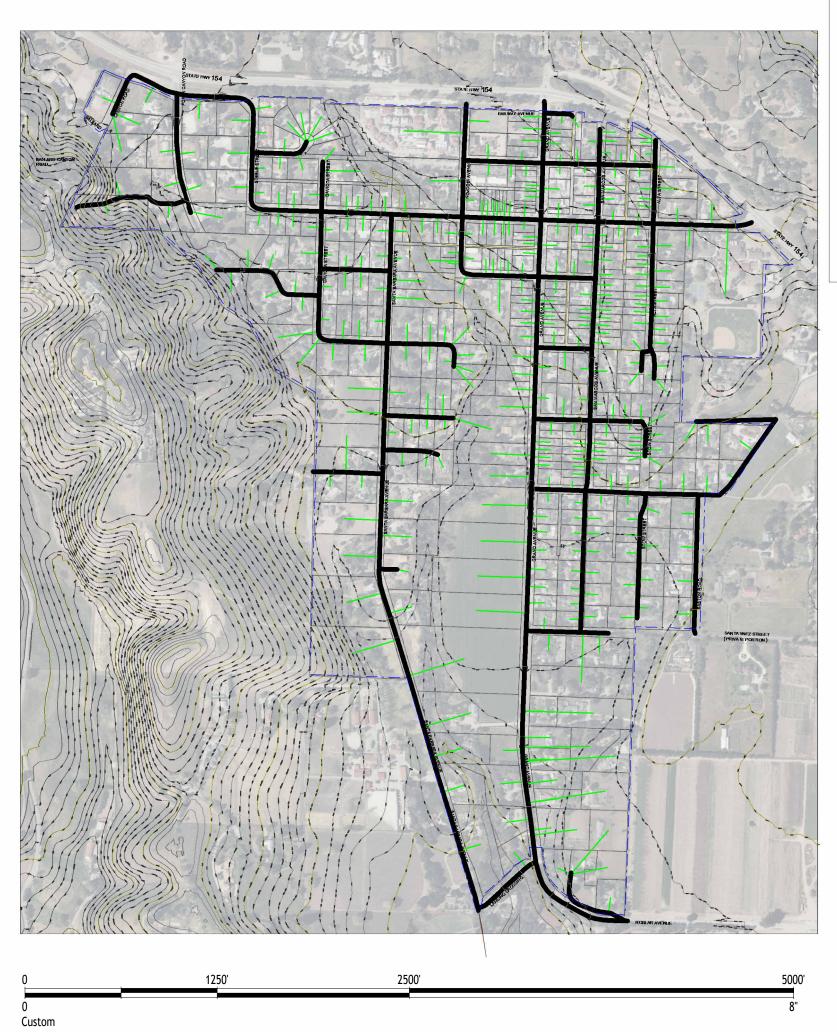
CIVI	<u>L</u>
	EXISTING ELEV.
	NEW ELEV.
y	EXISTING WATER
	EXISTING WATERMAIN
-W	NEW WATER
STS-	EXISTING STORM SEWER (ST
	NEW STORM SEWER (STS)
	EXISTING SANITARY SEWER (
—ss——	NEW SANITARY SEWER (SS)
PS	EXISTING PRESSURE SEWER
—PS-	NEW PRESSURE SEWER (PS)
	EXISTING FORCEMAIN (FM)
—FM	NEW FORCEMAIN (FM)
	KITCHEN WASTE LINE (KW)
-	NEW ELECTRICAL CONDUIT
	CLEANOUT
	EXISTING VALVE
	NEW VALVE
	EXISTING MANHOLE (MH)
	NEW MANHOLE (MH)

MECHANICAL

MECE	HANICAL	
1 1	UNION	
lόl	BALL VALVE	
N	CHECK VALVE	
5	PIPE BREAK	
Θ	PIPE RISE	
	PIPE DROP	
-	PIPE END CAP	

CONTACT INFO

ENGINEERING FIRM:	REGEN, PLLC
ADDRESS:	213 S 11TH STREET
	BOISE, ID 83702
PHONE:	(541) 580-2980
CONTACT:	TRISTIAN BOUNDS
OWNER:	LOS OLIVOS
ADDRESS:	LOS OLIVIOS
	SERVICE DISTRICT
PHONE:	(000) 000-0000





OVERALL SITE PLAN



PLAN SPECIFICATIONS

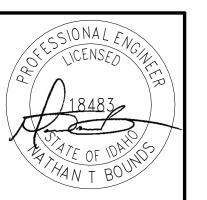
THESE PLANS DEPICT THE PRELIMINARY DESIGN OF THE WASTEWATER COLLECTION AND TREATMENT SYSTEM SERVICING THE SPRING ROCK DEVELOPMENT, LOCATED IN ADA COUNTY, IDAHO STATE. SPRING ROCK DEVELOMENT IS A PRIVATE COMMUNITY LOCATED ALONG TENMILE CREEK RD WITHIN THE CITY OF KUNA, ID.

THE SYSTEM DESIGN WILL ADHERE TO CALIFORNIA STATE AND SANTA BARBRA

TOPOGRAPHIC SURVEY NOTES:

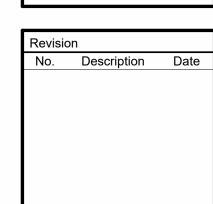
- LOCATION OF UNDERGROUND UTILITIES ARE APPROXIMATE ONLY AND ARE
- BASED ON RECORD INFORMATION. 2. ALL ELEVATIONS DISTANCES ARE IN FEET.
- 3. ELEVATIONS ARE REFERENCED TO MEAN SEA LEVEL.

COUNTY REQUIREMENTS AS PRESCRIBED IN CALIFORNIA.





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Project number	2203
Date	5.82
Drawn by	JS
Checked by	NTE

COVER SHEET

PRELIMINARY DESIGN

DRY UTILITIES NOTE:

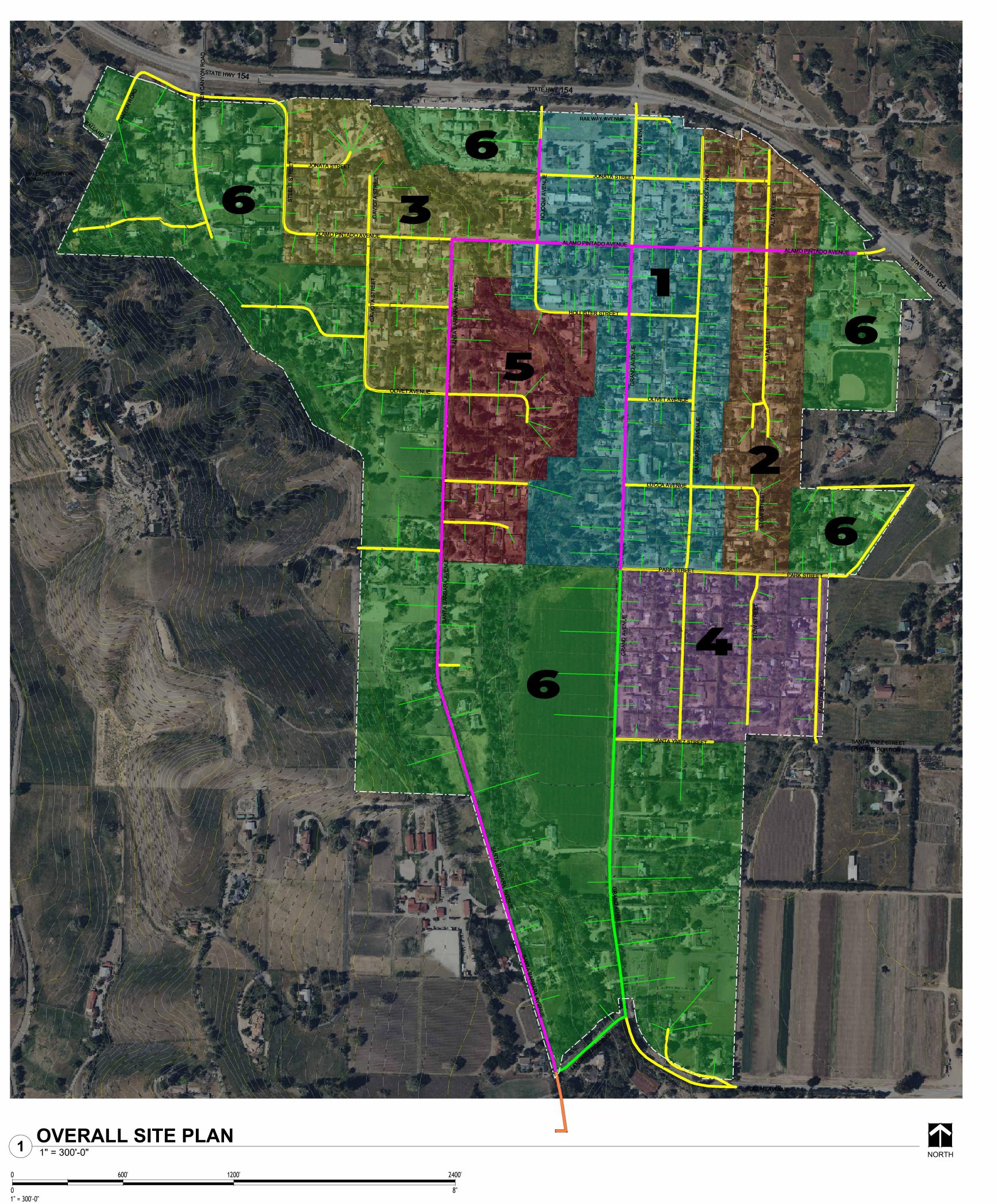
CONTACT:

DRY UTILITIES (ELECTRICITY, TELEPHONE, GAS, CABLE TV) SHOWN HEREON ARE APPROXIMATE. DESIGN SHALL BE BY THE SURVEYORS, AND INSTALLATION PAID FOR BY OWNER. CONTRACTOR SHALL COORDINATE WITH OWNER AND UTILITY COMPANIES IN THE TIMING AND INSTALLATION OF UTILITIES.

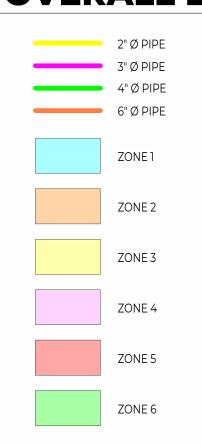
GUY SAVAGE

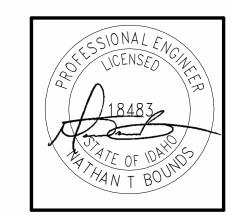
BEFORE YOU DIG, CALL





OVERALL LEGEND







LOS OLIVOS 30% DESIGN PLANS

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Revision

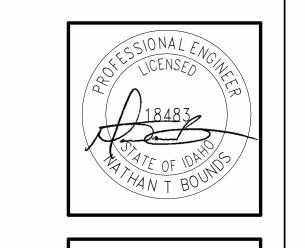
No. Description Date

Project number 22031
Date 5.824
Drawn by JS
Checked by NTB

PRELIMINARY DESIGN

SYSTEM OVERVIEW

C102





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Revision

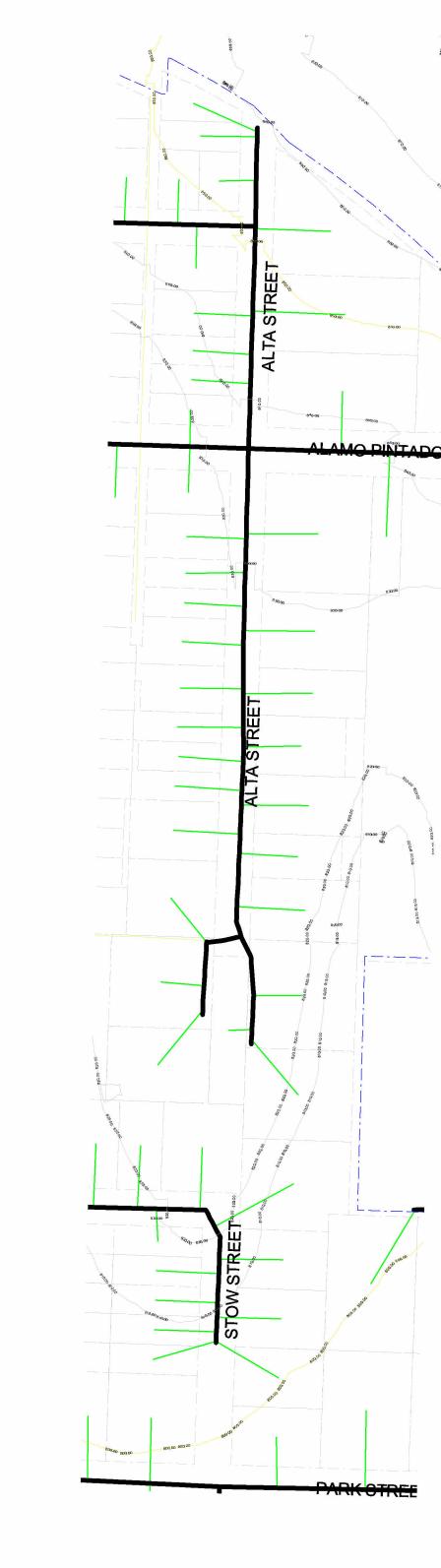
No. Description Date

5.824 Checked by

ZONES 1-3

PRELIMINARY DESIGN

C103

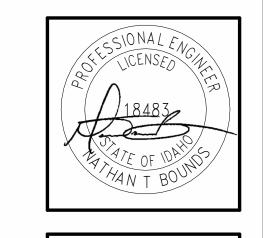




ZONE 11" = 160'-0"

RAILWAY AVENUE

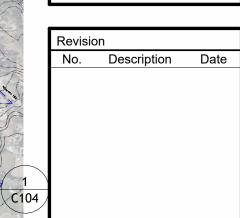
ZONE 21" = 160'-0"

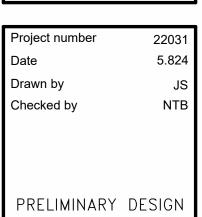




OS OLIVOS 30% DESIGN PLANS

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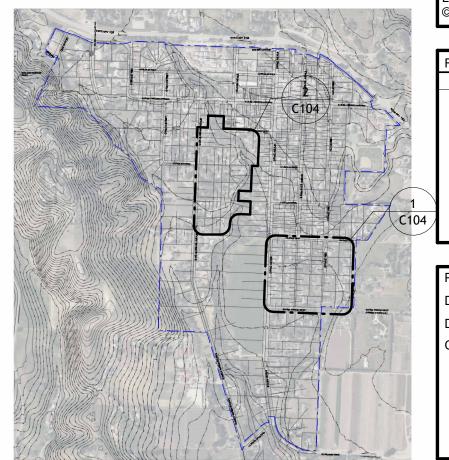


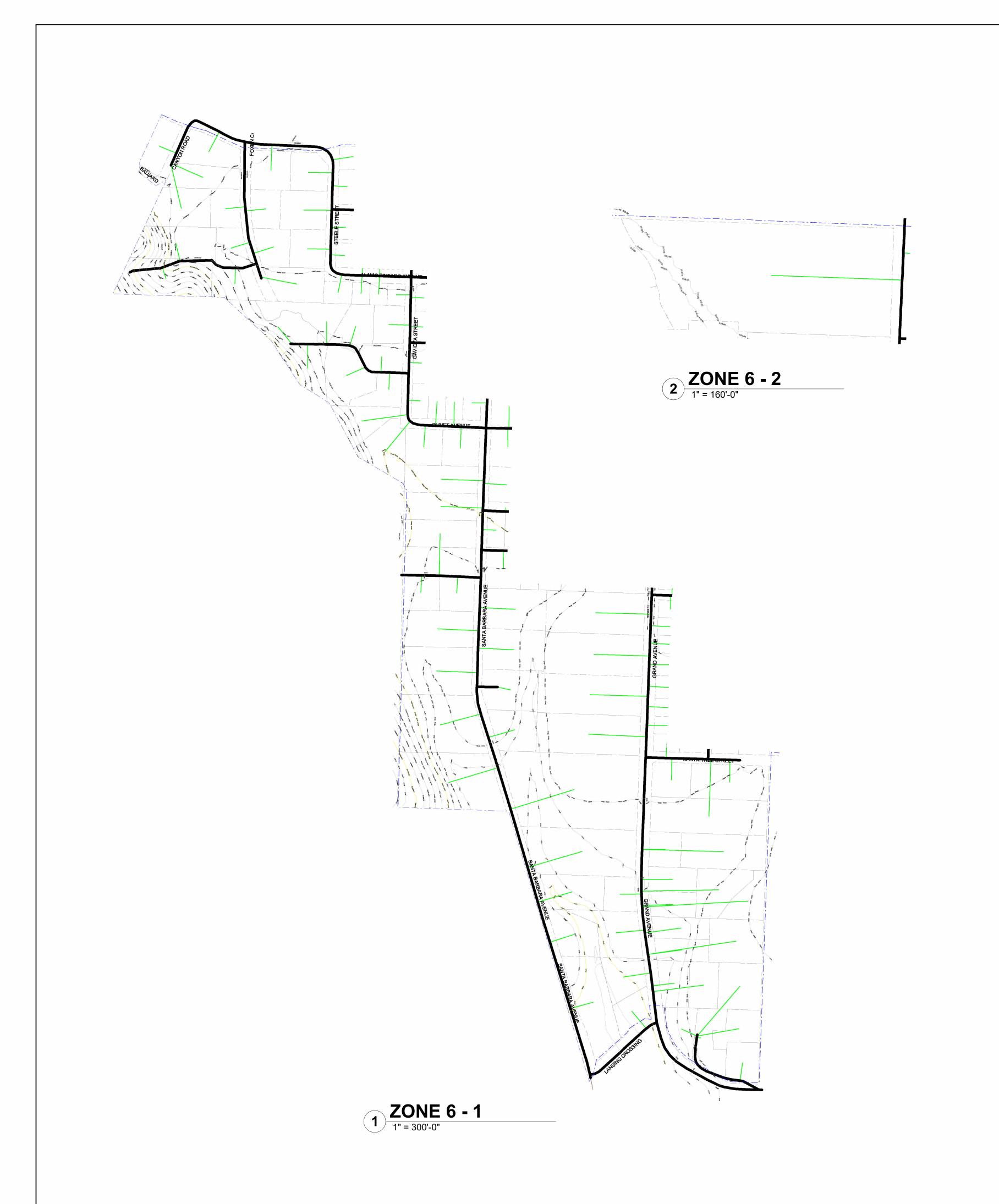
ZONES 4-5

C104



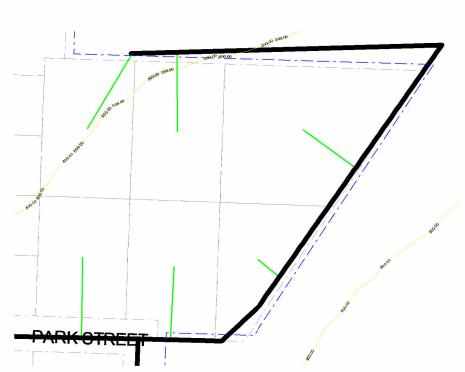




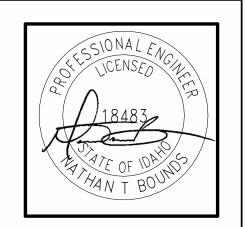








ZONE 6 - 41" = 160'-0"

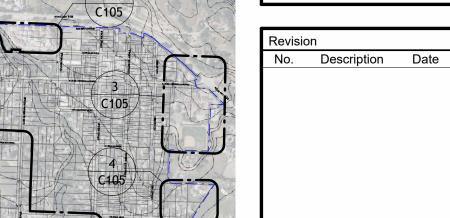




LOS OLIVOS 30% DESIGN PLANS

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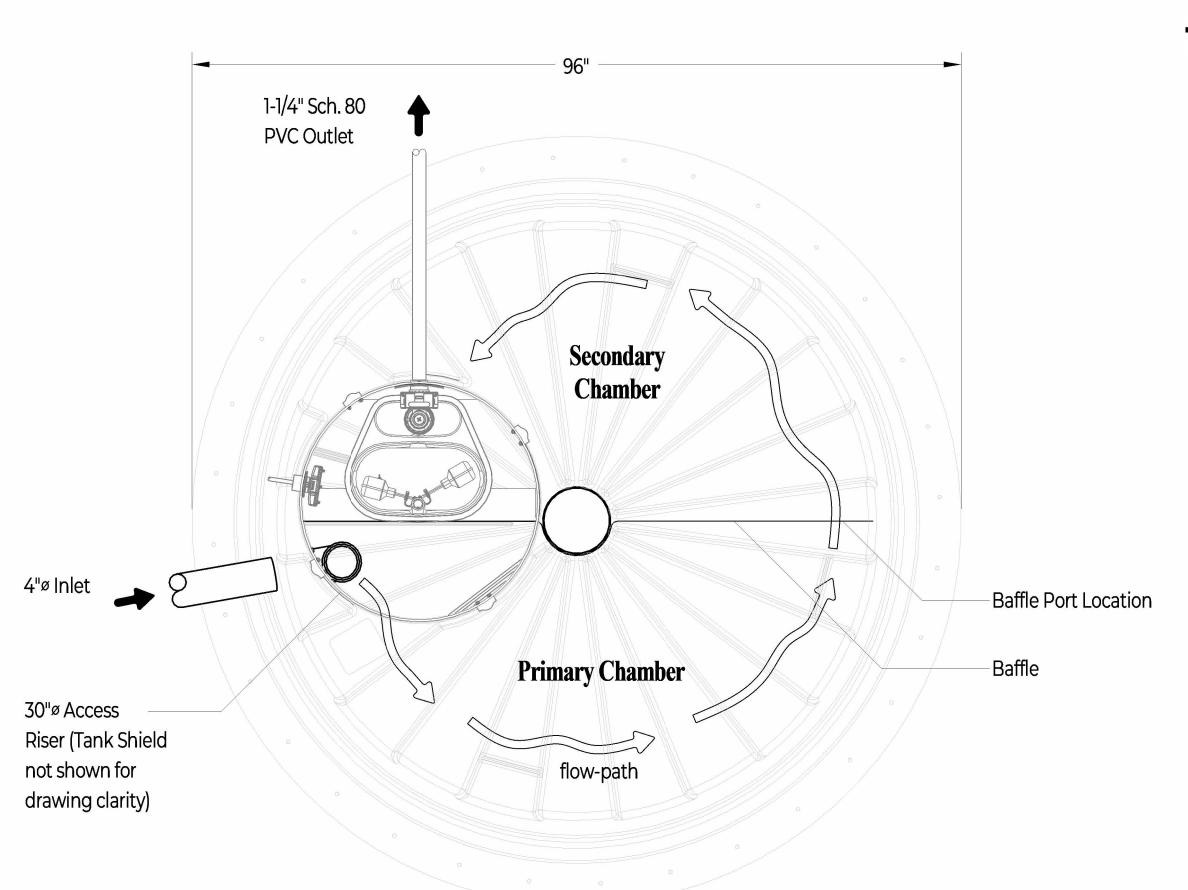


KEY PLAN

Project number	22031
Date	5.824
Drawn by	JS
Checked by	NTB
PRELIMINARY	DESIGN
I KEEHWIII VAKI	DESIGN

ZONE 6

C105



M1000 Tank Top Detail

Hanging Discharge

Assembly

TANK NOTES:

TANK VOLUMES: Total Volume: 1220 gal±

Nominal Volume: 1000 gal± @ 48"

Unit volume at typical <u>Operating Depth</u>: <u>20 gal./in.+</u>

LOADS: Top = 500 psf minimum

Lateral Load = 62.4 pcf, EFP Concentrated Wheel Load = 2500 lb.

The septic tank shall be capable of withstanding long-term hydrostatic loading, in addition to the soil loading, due to a water table maintained at ground surface.

Soil Bearing = 1000 psf (re-evaluate support base if soil bearing is less or unequal)

INSTALLATION: Installation, bedding, compaction, etc., shall be in "strict" compliance with the manufacturers standards and state or local rules and or guidelines. All tanks shall

be set level on a minimum 4 inch thick compacted sand or approved granular bedding overlying a firm uniform base. The base shall be stable and uniform in order to ensure equal bearing across the tank bottom. Installations with 18 inches or less of ground cover may require additional buoyancy considerations as

described in the manufacturers instructions. A minimum cover of 12 inches is required over the tank in areas subject to occasional light wheel loads. Refer to installation instructions

Document NIM-LOS-1.

CA

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No. Description Date

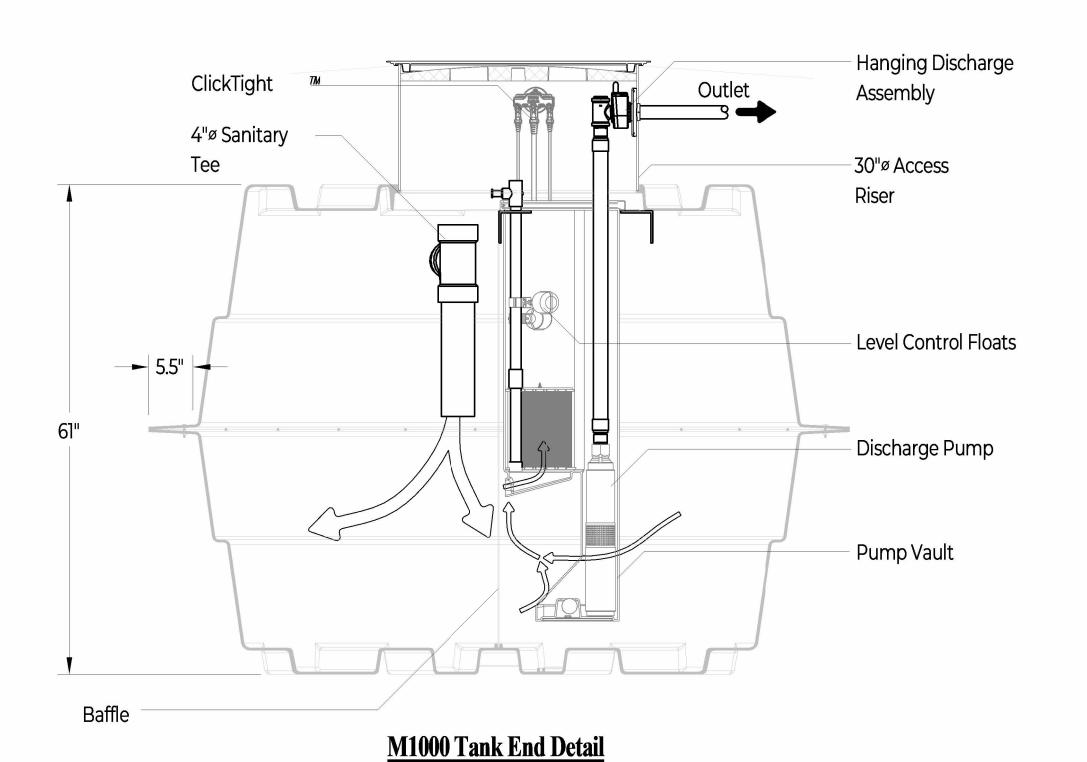
22031 5.824 Drawn by Checked by PRELIMINARY DESIGN

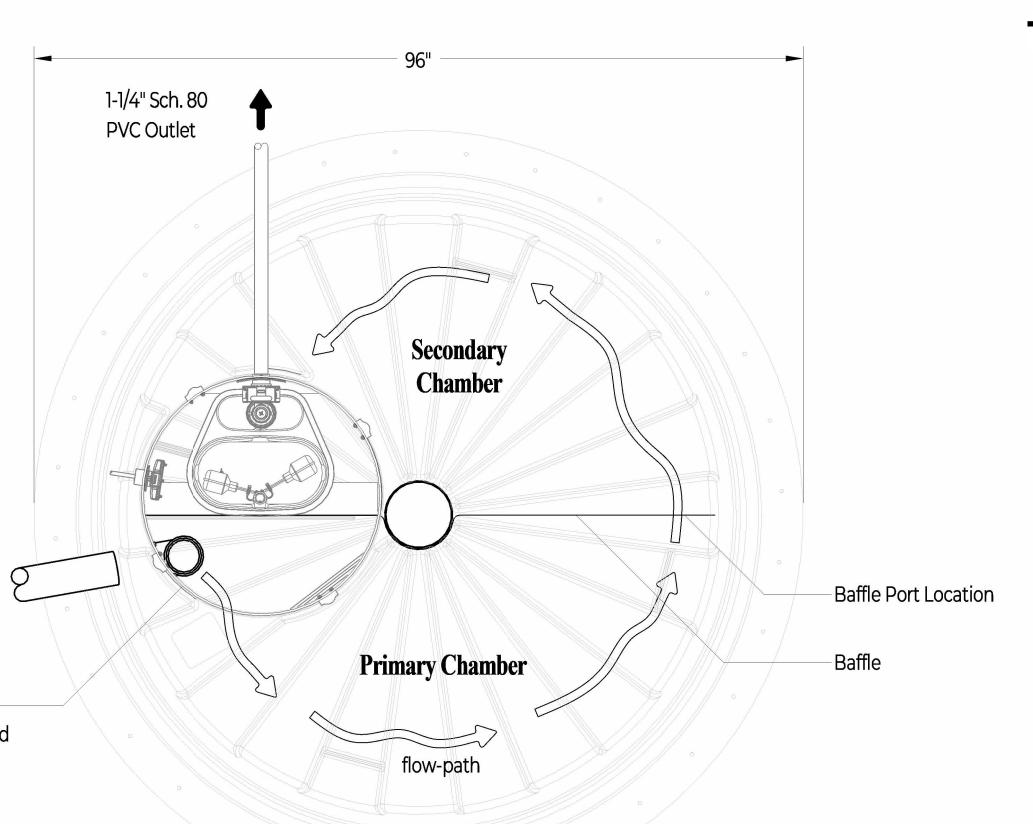
EXAMPLE OM LOT TANK **DETAILS**

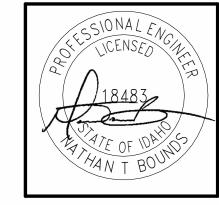
C106

Access Lid Slope (insulation optional) ClickTight ™ Finished Grade Inside 30"ø Access Riser Height Total (Burial depth less than 18" * 12" - 18" Min. Burial Depth with Tank Shield Gallons Inches requires Anti-flotation collar) Center Support Column 9.5"-54 1124 Baffle 42 881 36 744 и и 30 601 51.5" Baffle Port 24 460 (10"x22") 18 324 Pump Vault 12 200 6 83 M1000 Tank Side Detail

Graphic Scale





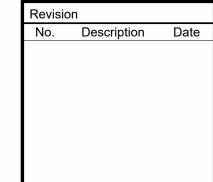


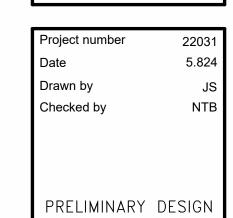


SOS ANS 30% DE

LOS OLIVOS, CA

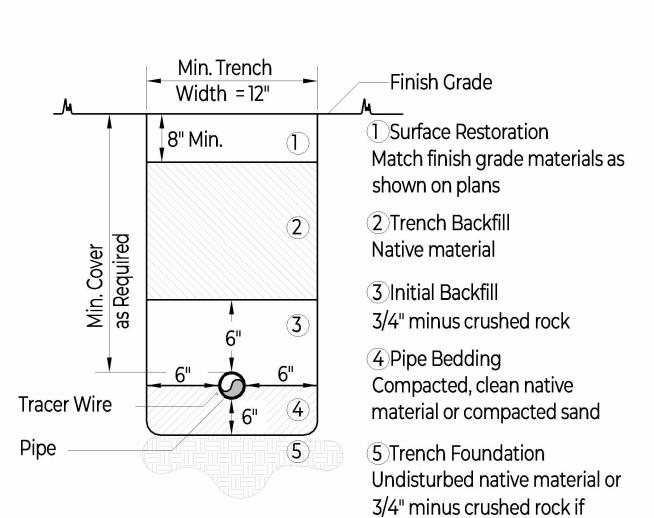
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EFFLUENT SEWER **DETAILS**

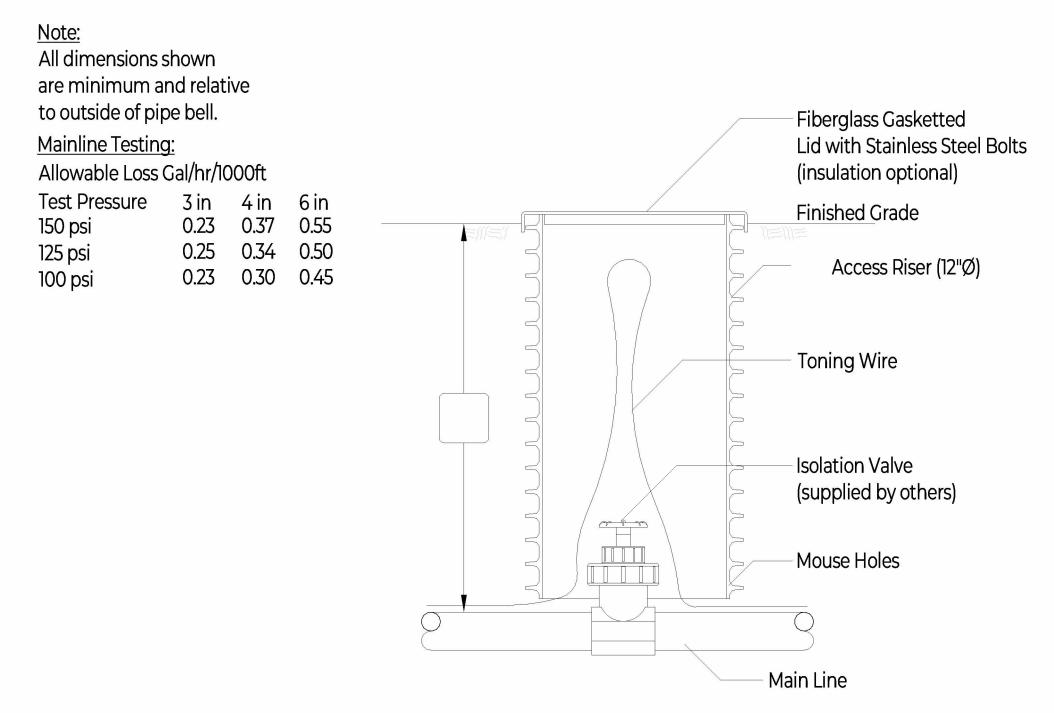
C107



Typical Sewer Trench Details

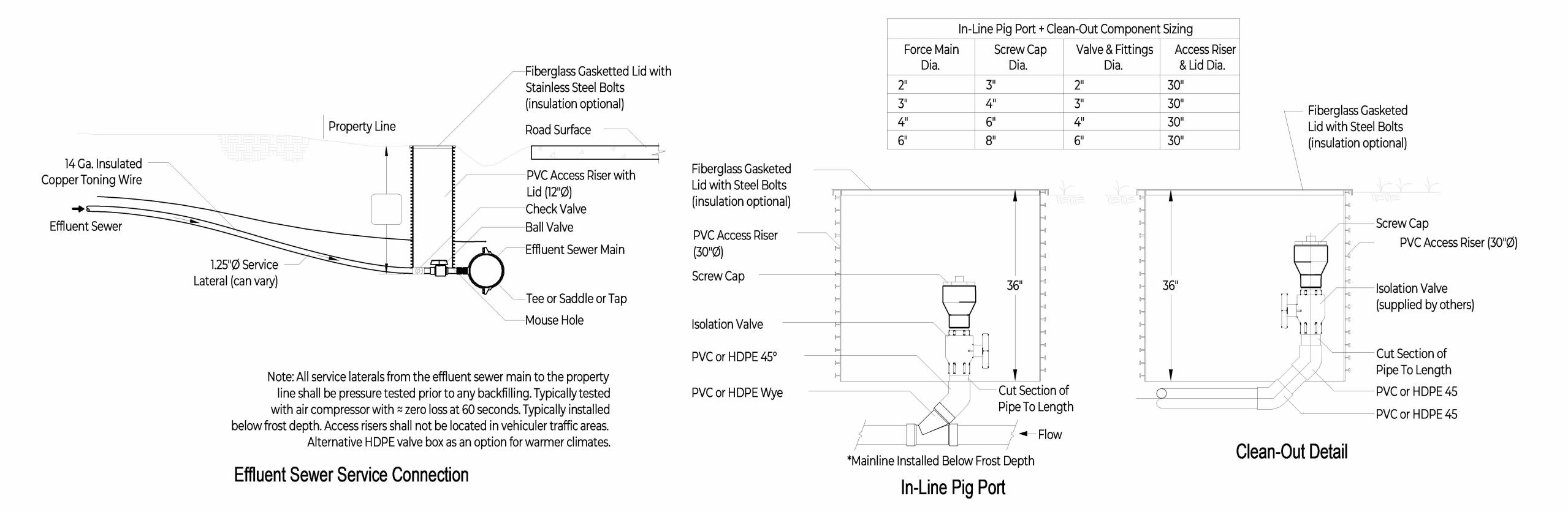
trench foundation stabilization

is required



Isolation Valve Detail

Air Valve Assembly - Manual Configuration



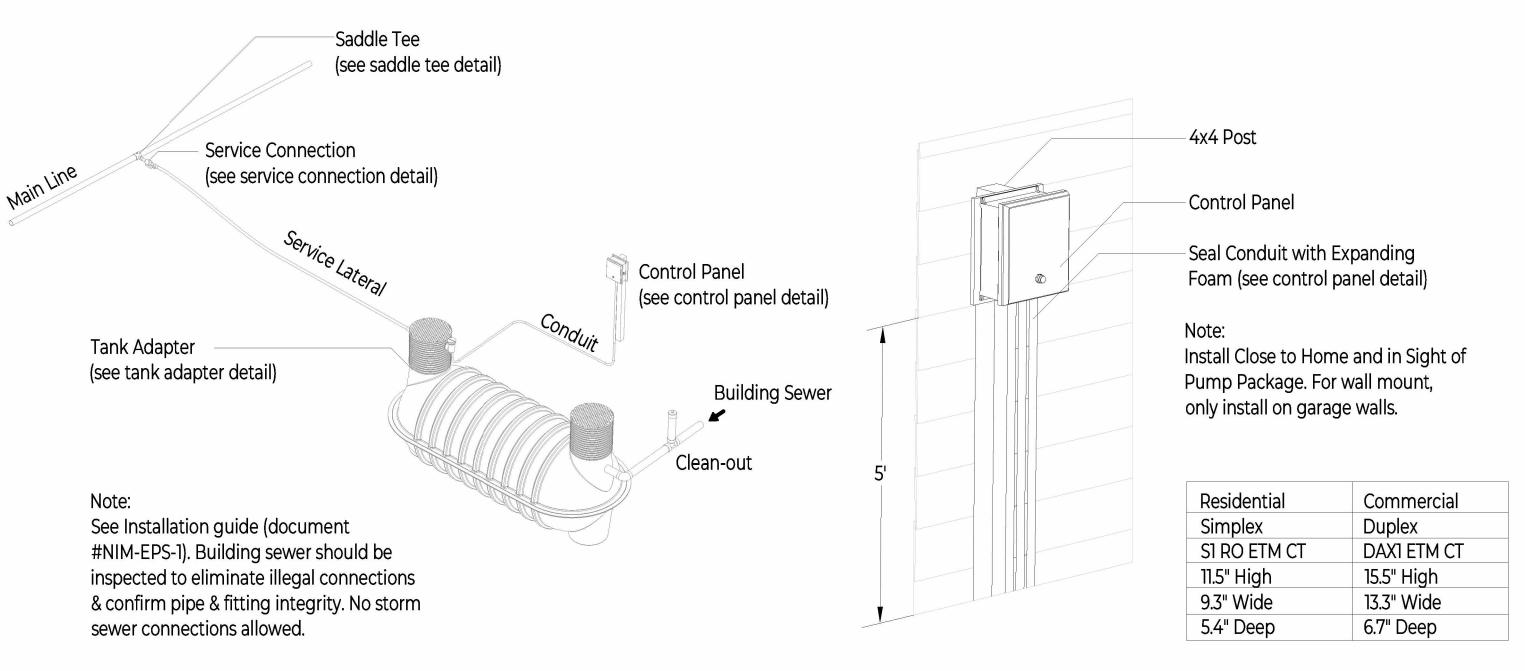
2" Isolation Ball Valve 30" (typ.) 3/4" Inspection Ball Valve 1/4" Pressure Gauge Port 7"-Air Valve Assembly Piping Drainrock Base Primary Connection Located at High Point

Secondary Connection Located Downstream

Vented Fiberglass Gasketed Lid with Stainless Steel Bolts and Carbon Filter

(insulation optional)

PVC Riser (30"Ø)



Typical Effluent Sewer Detail

Control Panel Detail

(Wall or 4x4 Post Mount)

18483 18483 18483 18483 18483 18483 18483 18483

EGEN

Service Lateral

Service Lateral

RECHITECTURE | ENGINEERING

Tee Saddle Detail

Hot Tap Saddle Detail

SS

OS OLIVOS, CA

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Revision

No. Description Date

Project number 22031
Date 5.824
Drawn by JS
Checked by NTB

PRELIMINARY DESIGN

EFFLUENT SEWER DETAILS

2108



APPENDIX B

Los Olivos Wastewater Collection Option A - Gravity Collection Zone 1 & 2, Effluent Sewer Zones 3-6

Item	Description	Unit	Quantity	Unit Price		Total Cost	Responsible Lead	Notes
Overhead	and Construction Support		 		 :			
1	Mobilization, Demobilization, Road Permits, Bonds, & Insurance	LS	1	\$ 2,000,000	\$	2,000,000	Unknown	Stantec Report
2	Construction Survey	LS	1	\$ 250,000	\$	250,000	Unknown	Stantec Report
3	Pothole Existing Utilities		1	\$ 250,000	\$	250,000	Unknown	Stantec Report
4	Traffic Control	LS LS	1	\$ 330,000	\$	330,000	Unknown	Should be unnecessasary for Effluent zones with boring
	Overhead and	l Construct	ion Support	Services Subtotal:	\$	2,830,000		
Gravity Co	llection System Construction							
Zone 1								
5	6-inch PVC Sewer Main (<15 ft cover)	LF LF EA	8000	\$ 180	\$	1,440,000	Unknown	Stantec Report
6	8-inch PVC Sewer Main (<15 ft cover)	LF	4000	\$ 240		960,000	Unknown	Stantec Report
8	48-inch Standard Manhole Service Connection	EA EA	30 149	\$ 17,500 \$ 12,000		525,000 1,788,000	Unknown Unknown	Stantec Report 4 in service connection (<14 ft) @ 50', landscape repair
	Service connection			nstruction Subtotal:	ر : \$	4,713,000	OTIKHOWII	4 in service connection (14 it) @ 30 , landscape repair
			· · · · · · · · · · · · · · · · · · ·		 			
Zone 2 9	6-inch PVC Sewer Main (<15 ft cover)	I E	900	\$ 180	ć	162,000	Unknown	Stantec Report
10	8-inch PVC Sewer Main (<15 ft cover)	LF LF	400	\$ 240		96,000	Unknown	Stantec Report
11	48-inch Standard Manhole	EA		\$ 17,500		140,000	Unknown	Stantec Report
12	48-inch Drop Manhole	EA EA	8 2	\$ 20,000	\$	40,000	Unknown	Stantec Report
13	Service Connection	EA	58	\$ 12,000	\$	696,000	Unknown	4 in service connection (<14 ft) @ 50', landscape repair
	Lift Station (duplex pumps, 350 gpm each, 25 HP	_						
14	each, 25 ft deep, 8 ft diameter, site gate, fence,	LS	1	\$ 600,000	\$	600,000	Unknown	Stantec Report
	electrical, SCE meter, backup power) 4-inch PVC Sewer Forcemain (5 ft cover,	L	ii		 			
15	separate trench)	LF	3,000	\$ 110	\$	330,000	Unknown	Stantec Report
	Separate denerg		Zone 2 Cor	nstruction Subtotal:	Ś	2,064,000		
								·
			Gravity (Collection Subtotal:	Ş	6,777,000		<u> </u>
Effluent S	ewer Collection System Construction							
Zone 3								
16	2-inch PVC Sewer Main (<4 ft cover)	LF	1705	\$ 51		86,955	Unknown	Direct boring estimate (Ventura Drilling)
17	3-inch PVC Sewer Main (<4 ft cover)	LF	1290	\$ 54	Ş	69,660	Unknown	Direct boring estimate (Ventura Drilling)
18 19	4-inch VC Sewer Main (<4 ft cover) Primary Treatment Tanks	LF EA	0 46	\$ 63 \$ 15,000	\$	690,000	Unknown Unknown	Direct boring estimate (Ventura Drilling) Primary tank, service connection, repair (Biosolutions)
	i i i i i i i i i i i i i i i i i i i		Zone 3 Cor			846,615	O I III I I I I I I I I I I I I I I I I	i
Zone 4 20	2-inch PVC Sewer Main (<4 ft cover)	LF	4133	\$ 51	خ	210,783	Unknown	Direct boring estimate (Ventura Drilling)
21	3-inch PVC Sewer Main (<4 ft cover)	LF	0	\$ 54	Ś	- 210,765	Unknown	Direct boring estimate (Ventura Drilling)
22	4-inch VC Sewer Main (<4 ft cover)	LF	900	\$ 63	\$	56,700	Unknown	Direct boring estimate (Ventura Drilling)
	Primary Treatment Tanks	EA	43	\$ 15,000		645,000	Unknown	Primary tank, service connection, repair (Biosolutions)
			Zone 4 Cor	nstruction Subtotal:	\$	912,483		<u></u>
Zone 5								
24	2-inch PVC Sewer Main (<4 ft cover)	LF	1200	\$ 51	\$	61,200	Unknown	Direct boring estimate (Ventura Drilling)
25	3-inch PVC Sewer Main (<4 ft cover)	LF	1321	\$ 54		71,334	Unknown	Direct boring estimate (Ventura Drilling)
26	4-inch VC Sewer Main (<4 ft cover)	LF	0	\$ 63			Unknown	Direct boring estimate (Ventura Drilling)
27	Primary Treatment Tanks	Each	26	\$ 15,000 nstruction Subtotal:	Ş . د	390,000	Unknown	Primary tank, service connection, repair (Biosolutions)
			ZUNE 3 COI	istruction Subtotal:	ڊ .	522,534		<u> </u>
Zone 6			 					
28	2-inch PVC Sewer Main (<4 ft cover)	LF	4494	\$ 51	\$	229,194	Unknown	Direct boring estimate (Ventura Drilling)
29 30	3-inch PVC Sewer Main (<4 ft cover) 4-inch VC Sewer Main (<4 ft cover)	LF LF	2214 2000	\$ 54 \$ 63		119,556 126,000	Unknown Unknown	Direct boring estimate (Ventura Drilling) Direct boring estimate (Ventura Drilling)
	Primary Treatment Tanks	Each	74	\$ 15,000	<u>ڊ</u> 5	1,110,000	Unknown	Primary tank, service connection, repair (Biosolutions)
				nstruction Subtotal:	; \$	1,584,750		7.5577
			ETTIUENT (Collection Subtotal:	; \$	3,866,382		L
		Co	llection Cor	struction Subtotal:	\$	13,473,382		
32	Construction Contingency (30%)		زِـــيــانِـا	·	\$	4,042,015		
	Col	iection Cor	struction &	Contingency Total	Ş	17,515,397		<u> </u>
	ng & Construction Management	,	,		Ţ			
33	Final Design Engineering & Support	LS LS	1	\$ 2,627,309			Unknown	Assume 15% of construction
34	Construction Management and Inspections		1	\$ 2,627,309		2,627,309	Unknown	Assume 15% of construction
	Engineerir	ig & Consti	ucuon Man	nagement Subtotal:	. <u>.</u> \$	5,254,619	L	L
		Opti	on #1 Collec	ction System Total:	\$	22,770,016		
Costs to h	e Provided by District				T			
35	Legal and Administration	LS	1	\$ 250,000	\$	250,000	District	Assumed
36	Property Acquisition for Lift Station	LS AC	0.4	\$ 1,000,000		400,000	District	Assume \$1M/Acre, 0.4 acres assumed
	Property Acquisition for Easements	AC	2.11	\$ 1,000,000			District	Assume \$1M/Acre, placeholder to match Stantec Estimate
			Dist	rict Costs Subtotal:	\$	2,760,000		
Ontion	#1 Collection System Total Costs:				\$	25,530,016		[
Option	ma conceiton system rutar custs.					, 23,330,010	i	<u>i</u>

Los Olivos Wastewater Collection Option B - Effluent Sewer Zones 1-6

Item Overbead	Description and Construction Support	Unit	Quantity	Unit Price	<u> </u>	Total Cost	Responsible Lead	Notes
overnead	Mobilization, Demobilization, Road Permits,	 I	T	 .	7		 	ļ
1	Bonds, & Insurance	LS	1	\$ 2,000,000	\$	2,000,000	Unknown	Stantec Report
2	Construction Survey	LS	1	\$ 250,000	\$	250,000	Unknown	Stantec Report
3	Pothole Existing Utilities	LS	1	\$ 250,000	\$	250,000	Unknown	Stantec Report
4	Traffic Control	LS	1	\$ 330,000	\$	330,000	Unknown	Should be unnecessasary for Effluent zones with boring
	Overhead an	d Construct	tion Suppor	t Services Subtotal:	\$	2,830,000		
Effluent S	ewer Collection System Construction							
Zone 1							,,	
5	2-inch PVC Sewer Main (<4 ft cover)	LF LF LF	5113	\$ 51		260,763	Unknown	Direct boring estimate
6	3-inch PVC Sewer Main (<4 ft cover)	LF	2989 0	\$ 54		161,406	Unknown	Direct boring estimate Direct boring estimate
7 8	4-inch PVC Sewer Main (<4 ft cover) Primary Treatment Tanks	Each	149	\$ 63 \$ 20,000		2,980,000	Unknown Unknown	Primary tank, service connection, landscape repair
	irilliary freatment rains	Lacii		nstruction Subtotal:	ر \$	3,402,169	OTIKITOWIT	rimary tank, service connection, landscape repair
	· · · · · · · · · · · · · · · · · · ·	·	·		ļ		ļ	}
Zone 2 9	2 inch DVC Sower Main (<4 ft cover)	10	2023	Ć E1	ċ	102 172	Unknown	Direct baring actimate
10	2-inch PVC Sewer Main (<4 ft cover) 3-inch PVC Sewer Main (<4 ft cover)	I F	700	\$ 51 \$ 54		103,173 37,800	Unknown Unknown	Direct boring estimate Direct boring estimate
11	4-inch PVC Sewer Main (<4 ft cover)	LF LF LF Each		\$ 63		-	Unknown	Direct boring estimate
12	Primary Treatment Tanks	Each	0 58	\$ 15,000	\$	870,000	Unknown	Primary tank, service connection, landscape repair
				nstruction Subtotal:	\$	1,010,973		
Zone 3	<u> </u>							
13	2-inch PVC Sewer Main (<4 ft cover)	LF	1705	\$ 51	\$	86,955	Unknown	Direct boring estimate
14	3-inch PVC Sewer Main (<4 ft cover)	LF LF	1290	\$ 54	\$	69,660	Unknown	Direct boring estimate
	4-inch PVC Sewer Main (<4 ft cover)		0	\$ 63			Unknown	Direct boring estimate
16	Primary Treatment Tanks	Each	46	\$ 15,000		690,000	Unknown	Primary tank, service connection, landscape repair
			Zone 3 Coi	nstruction Subtotal:	<u> </u>	846,615	<u>L</u>	<u> </u>
Zone 4			,				·	
17	2-inch PVC Sewer Main (<4 ft cover)	LF LF	4133	\$ 51	\$	210,783	Unknown	Direct boring estimate (Ventura Drilling)
	3-inch PVC Sewer Main (<4 ft cover)	LF LF	0 900	\$ 54		-	Unknown Unknown	Direct boring estimate (Ventura Drilling)
19 20	4-inch PVC Sewer Main (<4 ft cover) Primary Treatment Tanks	LF Each	43	\$ 63 \$ 15,000		56,700 645,000	Unknown	Direct boring estimate (Ventura Drilling) Primary tank, service connection, repair (Biosolutions)
	Trimary redeficite rains	Lucii		nstruction Subtotal:		912,483	Onknown	i milary tank, service connection, repair (biosolations)
7000 F	, , , , , , , , , , , , , , , , , , ,						·	······································
Zone 5 21	2-inch PVC Sewer Main (<4 ft cover)	I F	1200	\$ 51	Ś	61,200	Unknown	Direct boring estimate (Ventura Drilling)
22	3-inch PVC Sewer Main (<4 ft cover)	LF LF	1321	\$ 54		71,334	Unknown	Direct boring estimate (Ventura Drilling)
23	4-inch PVC Sewer Main (<4 ft cover)	LF LF		\$ 63	\$		Unknown	Direct boring estimate (Ventura Drilling)
24	Primary Treatment Tanks	Each	0 26	\$ 15,000	\$	390,000	Unknown	Primary tank, service connection, repair (Biosolutions)
			Zone 5 Co	nstruction Subtotal:	\$	522,534	 	
Zone 6								[
25	2-inch PVC Sewer Main (<4 ft cover)	LF	4494	\$ 51	\$	229,194	Unknown	Direct boring estimate (Ventura Drilling)
26	3-inch PVC Sewer Main (<4 ft cover)	LF LF	2214	\$ 54		119,556	Unknown	Direct boring estimate (Ventura Drilling)
27 28	4-inch PVC Sewer Main (<4 ft cover)	LF Early	2000 74	\$ 63 \$ 15,000		126,000	Unknown Unknown	Direct boring estimate (Ventura Drilling)
28	Primary Treatment Tanks	Each		nstruction Subtotal:	4	1,110,000 1,584,750	Unknown	Primary tank, service connection, repair (Biosolutions)
				·			<u> </u>	.,
			Effluent	Collection Subtotal:	\$	8,279,524	i 	<u> </u>
		Co	ollection Co	nstruction Subtotal:	\$	11,109,524	[
29	Construction Contingency (30%)]		\$	3,332,857		
		llection Co	nstruction 8	Contingency Total	\$	14,442,381	L	<u> </u>
Engineerii	ng & Construction Management				T		[
	Final Design Engineering & Support	LS	1	\$ 2,166,357	\$	2,166,357	Unknown	Assume 15% of construction
31	Construction Management and Inspections	LS	1	\$ 2,166,357	\$	2,166,357	Unknown	Assume 15% of construction
	Engineeri	ng & Const	ruction Mar	nagement Subtotal:	\$	4,332,714	<u> </u>	<u>i</u>
		Opti	on #2 Colle	ction System Total:	\$	18,775,096		[
Costs to !	o Brouided by District				7		*	·
Costs to b	e Provided by District Legal and Administration	LS	1	\$ 250,000	<	250,000	District	Assume
33	Property Acquisition for Lift Station	AC	0	\$ 250,000	Ś	-	District	Assume \$1M/Acre, Not needed with option #2
34	Property Acquisition for Easements	AC	2.11	\$ 1,000,000	\$	2,110,000	District	Assume \$1M/Acre, placeholder to match Stantec Estimate
			Dist	rict Costs Subtotal:		2,360,000		
Ontion	#2 Collection System Total Costs:				\$	21,135,096		[

Los Olivos Wastewater Collection Option C - Gravity Collection Zone 1 & 2, Effluent Sewer Zones 3-5, Alt Onsite Zone 6

				C - Gravity Colle				
Item Overhead	Description and Construction Support	Unit	Quantity	Unit Price	<u>L</u>	Total Cost	Responsible Lead	Notes
1	Mobilization, Demobilization, Road Permits, Bonds, & Insurance	LS	1	\$ 2,000,000	\$	2,000,000	Unknown	Stantec Report
2	Construction Survey	LS	1	\$ 250,000	\$	250,000	Unknown	Stantec Report
3	Pothole Existing Utilities	LS	1	\$ 250,000	\$	250,000	Unknown	Stantec Report
4	Traffic Control	LS	1	\$ 330,000	\$	330,000	Unknown	Should be unnecessasary for Effluent zones with boring
	Overhead and	l Construct	ion Support	Services Subtotal:	\$	2,830,000		
Gravity C	ollection System Construction							[
Zone 1					·			
5	6-inch PVC Sewer Main (<15 ft cover)	LF	8000	\$ 180		1,440,000	Unknown	Stantec Report
- 6 - 7	8-inch PVC Sewer Main (<15 ft cover) 48-inch Standard Manhole	LF EA	4000 30	\$ 240 \$ 17,500		960,000 525,000	Unknown Unknown	Stantec Report Stantec Report
8	Service Connection	EA EA	149	\$ 12,000		1,788,000	Unknown	4 in service connection (<14 ft) @ 50', landscape repair
				nstruction Subtotal:		4,713,000		14 in service connection (144 it) @ 30 , idiluscape repair
Zone 2	!				7			!
9	6-inch PVC Sewer Main (<15 ft cover)	LF	900	\$ 180	Ś	162,000	Unknown	Stantec Report
10	8-inch PVC Sewer Main (<15 ft cover)	LF	400	\$ 240		96,000	Unknown	Stantec Report
11	48-inch Standard Manhole	EA	8	\$ 17,500		140,000	Unknown	Stantec Report
12	48-inch Drop Manhole	EA	2	\$ 20,000		40,000	Unknown	Stantec Report
13	Service Connection	EA	58	\$ 12,000	\$	696,000	Unknown	4 in service connection (<14 ft) @ 50', landscape repair
14	Lift Station (duplex pumps, 350 gpm each, 25 HP each, 25 ft deep, 8 ft diameter, site gate, fence, electrical, SCE meter, backup power)	LS	1	\$ 600,000	\$	600,000	Unknown	Stantec Report
15	4-inch PVC Sewer Forcemain (5 ft cover,	LF	3,000	\$ 110	\$	330,000	Unknown	Stantec Report
	separate trench)			nstruction Subtotal:	.l	2,064,000		<u> </u>
							i	<u>i</u>
			Gravity (Collection Subtotal:	\$	6,777,000		<u> </u>
Effluent S	ewer Collection System Construction							
Zone 3	i		,		 -		r	
16 17	2-inch PVC Sewer Main (<4 ft cover) 3-inch PVC Sewer Main (<4 ft cover)	LF LF LF	1705 1290	\$ 51 \$ 54	\$	86,955 69,660	Unknown Unknown	Direct boring estimate (Ventura Drilling) Direct boring estimate (Ventura Drilling)
18	4-inch VC Sewer Main (<4 ft cover)	LF LF	0	\$ 54 \$ 63		69,660	Unknown	Direct boring estimate (Ventura Drilling) Direct boring estimate (Ventura Drilling)
19	Primary Treatment Tanks	Each	46	\$ 15,000		690,000	Unknown	Primary tank, service connection, repair (Biosolutions)
	A			struction Subtotal:	\$	846,615		
Zone 4								
20	2-inch PVC Sewer Main (<4 ft cover)	LF	4133	\$ 51	\$	210,783	Unknown	Direct boring estimate (Ventura Drilling)
21	3-inch PVC Sewer Main (<4 ft cover)	LF LF LF	0	\$ 54	\$	-	Unknown	Direct boring estimate (Ventura Drilling)
22	4-inch VC Sewer Main (<4 ft cover)		900	\$ 63	\$	56,700	Unknown	Direct boring estimate (Ventura Drilling)
23	Primary Treatment Tanks	Each	43	\$ 15,000 nstruction Subtotal:		645,000 912,483	Unknown	Primary tank, service connection, repair (Biosolutions)
			ZONE 4 COI	istruction subtotal.	۶ .	312,403	i 	<u>. </u>
Zone 5	2 inch DVG Course Marin (of the course)		4200		1-7	64 200	Unknown	Direct hading action to Direct on Delling)
24 25	2-inch PVC Sewer Main (<4 ft cover) 3-inch PVC Sewer Main (<4 ft cover)	LF LF	1200 1321	\$ 51 \$ 54		61,200 71,334	Unknown	Direct boring estimate (Ventura Drilling) Direct boring estimate (Ventura Drilling)
26	4-inch VC Sewer Main (<4 ft cover)	LF	2000	\$ 63	\$	126,000	Unknown	Direct boring estimate (Ventura Drilling)
27	Primary Treatment Tanks	Each	26	\$ 15,000	\$	390,000	Unknown	Primary tank, service connection, repair (Biosolutions)
			Zone 5 Cor	struction Subtotal:	\$	648,534		
			Effluent (Collection Subtotal:	\$	2,407,632		[
				struction Subtotal:				······································
28	Construction Contingency (30%)	CO	mection Cor	istruction Subtotal:	ڊ ڊ	12,014,632 3,604,390		
		lection Cor	nstruction &	Contingency Total	ب ا \$	15,619,022	·	
Zone 6	ļ	[I		1		l	
20ne 6 29	Individual Advanced Onsite Systems	LF	74	\$ 70,000	Ś	5,180,000	Unknown	Assumed TN requirement of 20 mg/L
		·		nstruction Subtotal:		5,180,000		
		Adus		Systems Subtotal:		5,180,000		[
		Auval	neu Oiisile	Jystems Subtolan	. 7		L	
30	Construction Contingency (30%)		. Callett	Camata :	\$	1,554,000		
	Adva	nced Onsit	e collection	Construction Total	, Ş	6,734,000	L	L
	ng & Construction Management				Ļ.,			
30 31	Final Design Engineering & Support Construction Management and Inspections	LS LS	1	\$ 2,342,853 \$ 2,342,853		2,342,853 2,342,853	Unknown Unknown	Assume 15% of construction (Advanced Onsite Not Included) Assume 15% of construction (Advanced Onsite Not Included)
٦1			ruction Man	ع کری :agement Subtotal		4,685,706	OTINIOWII	Advance 1976 of construction (Advanced Offsite Not Included)
	Option #3 Coll	ection & A	avanced Or	nsite System Total:	; \$	27,038,728		<u> </u>
	e Provided by District				ļ. <u>.</u>			
32	Legal and Administration	LS AC	1	\$ 250,000 \$ 1,000,000	\$ c	250,000 400,000	District District	Assumed
33 34	Property Acquisition for Lift Station Property Acquisition for Easements	AC AC	0.4 2.11	\$ 1,000,000	\$	400,000 2,110,000	District District	Assume \$1M/Acre, 0.4 acres assumed Assume \$1M/Acre, placeholder to match Stantec Estimate
54	roperty requisition for Lasements	AC	4 4	rict Costs Subtotal:		2,760,000	טוטנוננ	- 333e 92my reits, pracerioraer to materiotantee Estimate
- · · ·	#2.6 · · · · · · · · · · · · · · · · · · ·				·			
Option	#3 Gravity & Effluent Collection Sys	tems To	tal Cost	S:	\$	23,064,728	L	<u> </u>
Ontion	#3 Collection & Advanced Oncite Co	ctoms T	otal Coa	+c·	۲.	20 700 720	[
Option	#3 Collection & Advanced Onsite Sy	oceilis I	otal COS	L3.	\$	29,798,728	<u> </u>	<u> </u>

Los Olivos Wastewater Collection Option D - Effluent Sewer Zones 1-5, Alt Onsite Zone 6

Description ion Support Demobilization, Road Permits, urance Survey	Unit	Quantity	Unit Price	J	Total Cost	Responsible Lead	Notes
Demobilization, Road Permits, ırance	ıc	y					1
Survey	LJ	1	\$ 2,000,000	\$	2,000,000	Unknown	Stantec Report
	LS	1	\$ 250,000	\$	250,000	Unknown	Stantec Report
ing Utilities	LS	1	\$ 250,000	\$	250,000	Unknown	Stantec Report
ol .	LS	1	\$ 330,000	\$	330,000	Unknown	Should be unnecessasary for Effluent zones with boring
Overhead ar	d Construct	tion Support	Services Subtotal:	\$	2,830,000		
n System Construction							
wer Main (<4 ft cover)	LF	5113	\$ 51		260,763	Unknown	Direct boring estimate
ewer Main (<4 ft cover)	LF LF LF	2989	\$ 54	\$	161,406	Unknown	Direct boring estimate
ver Main (<4 ft cover)		0	\$ 63	\$	-	Unknown	Direct boring estimate
tment Tanks	Each	149	\$ 20,000 nstruction Subtotal:	Ş.	2,980,000 3,402,169	Unknown	Primary tank, service connection, landscape repair
		Zone 1 Cor	istruction Subtotal:	, <u>></u>	3,402,169		<u> </u>
				<u> </u>			
ewer Main (<4 ft cover)	LF LF LF Each	2023	\$ 51		103,173	Unknown	Direct boring estimate
ewer Main (<4 ft cover)	LF	700	\$ 54		37,800	Unknown	Direct boring estimate
ver Main (<4 ft cover)	LF	0 58	\$ 63		-	Unknown	Direct boring estimate
tment Tanks	Each		\$ 15,000		870,000	Unknown	Primary tank, service connection, landscape repair
		ZUIIE Z COI	nstruction Subtotal:	<u>; </u>	1,010,973	<u> </u>	<u>i</u>
ewer Main (<4 ft cover)	LF	1705	\$ 51		86,955	Unknown	Direct boring estimate
wer Main (<4 ft cover)	LF LF	1290	\$ 54	\$	69,660	Unknown	Direct boring estimate
ver Main (<4 ft cover)		0	\$ 63		-	Unknown	Direct boring estimate
tment Tanks	Each	46	\$ 15,000 nstruction Subtotal:		690,000 846,615	Unknown	Primary tank, service connection, landscape repair
		ZUITE 3 CUI	istruction subtotal.	٠, ٠	640,013	L	<u>i</u>
		·				r	
wer Main (<4 ft cover)	LF	4133	\$ 51		210,783	Unknown	Direct boring estimate
ewer Main (<4 ft cover)	LF LF	0	\$ 54 \$ 63		- 56 700	Unknown	Direct boring estimate
ver Main (<4 ft cover) tment Tanks	Each	900 43	\$ 63 \$ 15,000		56,700 645,000	Unknown Unknown	Direct boring estimate Primary tank, service connection, landscape repair
ment ranks	Eacii		nstruction Subtotal:		912,483	UTIKITOWIT	Filliary tank, service connection, landscape repair
·				1		L	
		4200			64 200		
ewer Main (<4 ft cover)	LF	1200 1321	\$ 51 \$ 54		61,200	Unknown	Direct boring estimate
ewer Main (<4 ft cover) ver Main (<4 ft cover)	LF LF	2000	\$ 54	ç	71,334 126,000	Unknown Unknown	Direct boring estimate Direct boring estimate
tment Tanks	Each	26	\$ 15,000		390,000	Unknown	Primary tank, service connection, landscape repair
			nstruction Subtotal:		648,534		
		Effluent	Collection Subtotal:	<u> </u>	6,820,774	L	<u> </u>
		Moction Co.	nstruction Subtotal:	ė	9,650,774	r	
Contingency (30%)	T	mection coi	istruction subtotal.		2,895,232		
	llection Co	nstruction 8	Contingency Total	Ś	12,546,006		
				JT.		\	
		1			5 400 000		
vanced Onsite Systems	LF	74	\$ 70,000 nstruction Subtotal:		5,180,000 5,180,000	Unknown	Assumed TN requirement of 20 mg/L
				J'		L	L
	Adva	nced Onsite	Systems Subtotal:	\$	5,180,000		
Contingency (30%)	·····	Ţ		Ś	1,554,000		
sortangency (50%)	Adva	nced Onsite	Construction Total	\$	6,734,000		
						·	r
ion Management	1 10		ć 1004.001	 _	1 004 004	Under	Assume AFR/ of construction /Advanced Outlie Nove 1
Engineering & Support Management and Inspections	LS IS	1	\$ 1,881,901 \$ 1,881,901		1,881,901 1,881,901	Unknown Unknown	Assume 15% of construction (Advanced Onsite Not Include Assume 15% of construction (Advanced Onsite Not Include
	ng & Const	ruction Mar	agement Subtotal:		1,881,901 3,763,802	OHMIUWII	2334 TO 1378 OF CONSTRUCTION (Advanced Offsite NOT INCIDITE
						·	
Option #4 Co	llection & A	dvanced O	nsite System Total:	\$	23,043,808	L	
District				;			
ministration	LS	1	\$ 250,000	\$	250,000	District	Assumed
uisition for Lift Station	AC	0.4	\$ -	\$	-	District	Assume \$1M/Acre, Not needed option #4
uisition for Easements	LS AC AC	2.11	\$ 1,000,000	\$	2,110,000	District	Assume \$1M/Acre, placeholder to match Stantec Estimate
		Dist	rict Costs Subtotal:		2,360,000		
t Commercial Collection To the	C4-				40.663.33		
t Sewer Collection Total	Losts:			Ş	18,669,808		<u>[</u>
		er Collection Total Costs: er Collection & Advanced C		er Collection Total Costs: er Collection & Advanced Onsite Total Costs:			